

**EFFECT OF SOIL VARIABILITY ON THE
BEARING CAPACITY OF FOOTINGS ON
MULTI-LAYERED SOIL**

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REFERENCES

- Abbo, A. J. and Sloan, S. W. (1995).** A Smooth Hyperbolic Approximation to the Mohr-Coulomb Yield Criterion. *Computers on Structures*, Vol. 54, No. 3, pp. 427–441.
- Agterberg, F. P. (1970).** Autocorrelation Function in Geology, In *Geostatistics - A Colloquium*, Merriam, D. F. (ed.), Plenum Press, New York, pp. 113-141.
- Amari, S. I., Murata, N., Muller, K. R., Finke, M. and Yang, H. H. (1997).** Asymptotic Statistical Theory of Overtraining and Cross-validation. *IEEE Transactions on Neural Networks*, Vol. 8, No.5, pp. 985–996.
- Anderheggen, E. and Knopfel, H. (1972).** Finite Element Limit Analyses Using Linear Programming, *International Journal of Solids and Structures*, Vol. 8, pp. 1413–1431.
- Ang, A. H. -S. and Tang, W. H. (1984).** Probability Concepts in Engineering Planning and Design. In: Risk and Reliability, Vol. II. Willey, New York.
- Baecher, G. B. (1982).** Simplified Geotechnical Data Analysis. *Proc. of the NATO Advanced Study Institute on Reliability Theory & its Appl'n in Structural & Soil Mechanics*, Bornholm, Denmark, Martinus Nijhoff (Publ. 1983), pp. 257–277.
- Baecher, G. B. and Christian, J. T. (2003).** *Reliability and Statistics in Geotechnical Engineering*. John Wiley & sons, New York, 605 p.
- Baracos, A. (1957).** The Foundation Failure of the Transcona Grain Elevator. The Engineering Journal, Vol. 40, No. 7 (July).
- Berke, L. and Hajela, P. (1991).** Application of Neural Networks in Structural Optimization. *NATO/AGARD Advanced Study Institute*, Vol. 23, No. I-II, pp. 731–745.

- Bottero, A., Negre, R., Pastor, J. and Turgeman, S. (1980).** Finite Element Method and Limit Analysis Theory for Soil Mechanics Problems. *Computational Methods in Applied Mechanics and Engineering*, Vol. 22, pp. 131–149.
- Bowden, G. J., Maier, H. R. and Dandy, G. C. (2002).** Optimal Division of Data for Neural Network Models in Water Resources Applications. *Water Resources Research*, Vol. 38, No. 2, pp. 1–11.
- Bowles, J. E. (1988).** *Foundation Analysis and Design*, Fourth Edition, McGraw-Hill, New York.
- Bowles, J. E. (1996).** *Foundation Analysis and Design*, Fifth Edition, McGraw-Hill, United States of America. 1175 p.
- Braddock, R. D., Kremmer, M. L. and Sanzogni, L. (1998).** Feed-forward Artificial Neural Network Model for Forecasting Rainfall Run-off. *Environmetrics*, Vol. 9, pp. 419–432.
- Breiman, L. (1994).** Comment on 'Neural Networks: A Review from a Statistical' by B. Cheng and D. M. Titterington. *Statistical Science*, Vol. 9, No. 1, pp. 38–42.
- Brinch Hansen, J. (1970).** A Revised and Extended Formula for Bearing Capacity, Danish Geotechnical Institute, *Bulletin 28*, Copenhagen.
- Brocklehurst, C. J. (1993).** Finite Element Studies of Reinforced and Un-reinforced Soil System. *Computers and Geotechnics*, Vol. 7, pp. 19–36.
- Brown, J. D. and Meyerhof, G. G. (1969).** Experimental Study of Bearing Capacity in Layered Clays. *Proceedings of the 7th International Conference on Soil Mechanics and Foundation Engineering*, Vol. 2, pp. 45–51.
- Brown, M. and Harris, C. J. (1995).** A Perspective and Critique of Adaptive Neurofuzzy Systems used for Modelling and Control Applications. *Int. J. Neural Systems*, Vol. 6, No. 2, pp. 197–220.

- Burd, H. J. and Frydman, S. (1996).** Discussion on Bearing Capacity of Footings over Two-Layer Foundation Soils. *Journal of Geotechnical Engineering*, ASCE, Vol. 122, No. 8, pp. 699–700.
- Burd, H. J. and Frydman, S. (1997).** Bearing Capacity of Plane Strain Footings on Layered Soils. *Canadian Geotechnical Journal*, Vol. 34, pp. 241–253.
- Burke, L. I. and Ignizio, J. P. (1992).** Neural Networks and Operations Research: An Overview. *Computer and Operations Research*, Vol. 19, No. 3, pp. 179–189.
- Button, S. J. (1953).** The Bearing Capacity of Footings on a Two-Layer Cohesive Subsoil. *Proceeding 3rd International Conference on Soil Mechanic and Foundation Engineering*, Zurich, Switzerland, Vol. 1, pp. 332–335.
- Campanella, R. G., Wickremesinghe, D. S. and Robertson, P. K. (1987).** Statistical Treatment of Cone Penetrometer Test Data. *Proc. 5th Int. Conf. on Applications of Statistics and Probability in Soil and Struct. Engrg.*, Vancouver, pp. 1011–1019.
- Campolo, M., Soldati, A. and Andreussi, P. (1999).** Forecasting River Flow Rate During Low-flow Periods Using Neural Networks. *Water Resources Research*, Vol. 35, No. 11, pp. 3547–3552.
- Carter, J. P., Desai, C. S., Potts, D. M., Schweiger, H. F. and Sloan, S. W. (2000).** Computing and Computer Modelling in Geotechnical Engineering. *GeoEng2000*, Melbourne, Australia, Lancaster, Pa. USA: Technomic Publishing Co., pp. 1157–1252.
- Carpenter, L. (1980).** Computer Rendering of Fractal Curves and Surfaces. *SIGGRAPH*, Association for Computing Machinery, Seattle, Washington, USA.
- Caudill, M. (1988).** Neural Network Primer, Part III. *AI Expert*, Vol. 3, No. 6, pp. 53–59.

- Chan, L. W. and Fallside, F. (1987).** An Adaptive Training Algorithm for Backpropagation Networks.” *Computer Speech and Language*, Vol. 2, pp. 205–218.
- Chatfield, C. (1975).** *The Analysis of Time Series: Theory and Practice*, Chapman and Hall, London, 263 p.
- Chen, W. F. (1975).** *Limit Analysis and Soil Plasticity*. Amsterdam: Elsevier. 638p.
- Chen, W. F. and Dividson H. L. (1973).** Bearing Capacity Determination by Limit Analysis. *Journal of Soil Mechanics and Foundation Division*. ASCE. Vol. 99, pp. 443–449.
- Chen, W. F. and McCarron, W. O. (1991).** Bearing Capacity of Shallow Foundations. Chapter 4, *Foundation Engineering Handbook*, Second Edition, H. Y. Fang (ed.), Van Nostrand Reinhold, New York, pp. 144–165.
- Chen W.F., and Mizuno, E. (1990).** *Nonlinear Analysis in Soil Mechanics (Developments in Geotechnical Engineering)*. Elsevier, Amsterdam.
- Chester, D. L. (1990).** Why Two Hidden Layers are Better Than One. *Int. Joint Conf. on Neural Networks*, Vol. 1, pp. 265–268.
- Coduto, D. P. (2001).** *Foundation Design and Practices*, Second Edition, Prentice Hall.
- Cybenko, G. (1989).** Approximation by Superpositions of a Sigmoidal Function. *Mathematics of Control, Signals and Systems*, Vol. 3, pp. 303–314.
- Das, B. M. (1997).** *Principles of Foundation Engineering*, Fourth Edition, Pws Publishing Co.
- Davis, E. H. and Booker, J. R. (1973).** The Effect of Increasing Strength with Depth on the Bearing Capacity of Clays. *Géotechnique*, Vol. 23, No. 4, pp. 551–563.

- Drucker, D. C. (1950).** Some Implications of Work Hardening and Ideal Plasticity. *Quarterly Journal of Applied Mathematics*, Vol. 7, No. 4, pp. 411–418.
- Drucker, D. C., Greenberg, H. J. and Prager, W. (1951).** The Safety Factor of an Elastic-plastic Body in Plane Strain. *J. Appl. Mech. Trans.*, ASME, Vol. 73, pp. 371–378.
- Drucker, D. C., Prager, W. and Greenberg, H. J. (1952).** Extended Limit Design Theorems for Continuous Media. *Quarterly Journal of Applied Mathematics*, Vol. 9, pp. 381–389.
- El-Ramly, H., Morgenstern, N. R., and Cruden, D. (2002).** Probabilistic Slope Stability Analysis for Practice. *Canadian Geotechnical Journal*, Vo. 39, pp. 665–683.
- Fahlman, S. E. (1988).** Fast Learning Variations on Back-propagation: An Empirical Study. *Proc. 1988 Connectionist Models Summer School*, Pittsburgh, pp. 38–51.
- Faraway, J. and Chatfield, C. (1998).** Time Series Forecasting with Neural Networks: A Comparative Study Using the Airline Data. *Applied Statistics*, Vol. 47, No. 2, pp. 231–250.
- Fausett, L. V. (1994).** *Fundamentals Neural Networks: Architecture, Algorithms, and Applications*, Prentice-Hall, Englewood Cliffs, New Jersey.
- Fenton, G. A. (1990).** *Simulation and Analysis of Random Fields*, PhD Thesis, Princeton University, New Jersey, USA.
- Fenton, G. A. (1994).** Error Evaluation of Three Random Field Generators. *Journal of Engineering Mechanics*, Vol. 120, No. 12, pp. 2487–2497.
- Fenton, G. A. (1999).** Estimation for Stochastic Soil Models. *Journal of Geotechnical and Geoenvironmental Engineering*, ASCE, Vol. 125, No. 6, pp. 470–485.

- Fenton, G. A. and Griffiths, D. V. (2002).** Probabilistic Foundation Settlement on Spatially Random Soil. *Journal of Geotechnical and Geoenvironmental Engineering*, ASCE, Vol.128, No.5, pp. 381–390.
- Fenton, G. A. and Griffiths, D. V. (2003).** Bearing-Capacity Prediction of Spatially Random c - ϕ Soil. *Canadian Geotechnical Journal*, Vol. 40, pp. 54–65.
- Fenton, G. A. and Vanmarcke, E. H. (1990).** Simulation of Random Fields via Local Average Subdivision. *Journal of Engineering Mechanics*, Vol. 116, No. 8, pp. 1733–1749.
- Fenton, G. A., Zhou, H., Jaksa, M. B. and Griffiths, D. V. (2003).** Reliability Analysis of a Strip Footing Designed Against Settlement. *Applications of Statistics and Probability in Civil Engineering, ICASP9*, A. Der Kiureghian, S. Madanat and J. M. Pestana (eds.), San Francisco, Millpress, Rotterdam, Vol. 2, pp. 1271–1277.
- Flood, I. (1991).** A Gaussian-based Neural Network Architecture and Complementary Training Algorithm. *Proc. Int. Conf. on Neural Networks*, New York, Vol. 1, pp. 171–176.
- Flood, I. and Kartam, N. (1994).** Neural Networks in Civil Engineering I: Principles and Understanding. *J. Computing in Civil Eng.*, Vol. 8, No. 2, pp. 131–148.
- Florkiewicz, A. (1989).** Upper Bound to Bearing Capacity of Layered Soils. *Canadian Geotechnical Journal*, Vol. 16, pp. 730–736.
- Fortin, V., Ouarda, T. B. M. J. and Bobee, B. (1997).** Comment on 'The Use of Artificial Neural Networks for the Prediction of Water Quality Parameters' by H. R. Maier and G. C. Dandy. *Water Resources Research*, Vol. 33, No. 10, pp. 2423–2424.
- Fournier, A., Fussell, D. and Carpenter, L. (1982).** Computer Rendering of Stochastic Models, *Communications of the ACM*, Vol. 25, No. 6, pp. 371–384.

- Ghaboussi, J. and Sidarta, D. E. (1998).** New Nested Adaptive Neural Networks (NANN) for Constitutive Modeling. *J. Computers and Geotechnics*, Vol. 22, No. 1, pp. 29–52.
- Ghosh, S. and Kikuchi, N. (1991).** An Arbitrary Lagrangian–Eulerian Finite Element Method for Large Deformation Analysis of Elastic-Viscoplastic Solid. *Computer Methods in Applied Mechanics and Engineering*, Vol. 86, pp. 127–188.
- Goh, A. T. C. (1994b).** Seismic Liquefaction Potential Assessed by Neural Network. *Journal of Geotechnical and Geoenvironmental Engineering*, ASCE, Vol. 120, No. 9, pp. 1467–1480.
- Griffiths, D. V. (1982a).** Computation of Bearing Capacity on Layered Soils. *Proceedings 4th International Conference on Numerical Methods in Geomechanics*. Vol. 1, pp. 163–170.
- Griffiths, D. V. (1982b).** Computation of Bearing Capacity Factors Using Finite Elements. *Géotechnique*, Vol. 32, No. 3, pp. 195–202.
- Griffiths, D. V. and Fenton, G. A. (2001).** Bearing Capacity of Spatially Random Soil: The Undrained Clay Prandtl Problem Revisited. *Geotechnique*, Vol. 51, no.4, pp.351–359.
- Griffiths, D. V., Fenton, G. A. and Manoharan, N. (2002).** Bearing Capacity of a Rough Rigid Strip Footing on Cohesive Soil: A Probabilistic Study. *Journal of Geotechnical and Geoenvironmental Engineering*, ASCE, Vol. 128, No. 9, pp. 743–755.
- Hammerstrom, D. (1993).** Working with Neural Networks. *IEEE Spectrum*, Vol. 30, No. 7, pp. 46–53.
- Hanna, A. M. and Meyerhof, G. G. (1980).** Design Charts for Ultimate Bearing Capacity of Foundations on Sand Overlying Soft Clay. *Canadian Geotechnical Journal*, Vol. 17, pp. 300–303.

- Hassoun, M. H. (1995).** *Fundamentals of Artificial Neural Networks*, MIT Press, Cambridge.
- Head, K. H. (1997).** *Experimental soil mechanics*, Prentice Hall.
- Hegazy, T., Fazio, P. and Moselhi, O. (1994).** Developing Practical Neural Network Applications using Back-propagation. *Microcomputers in Civil Engrg*, Vol. 9, pp. 145–159.
- Hertz, J. A., Krogh, A., and Palmer, R. G. (1991).** *Introduction to the Theory of Neural Computation*, Addison-Wesely Publishing Company, Red Wood City, California.
- Hecht-Nielsen, R. (1987).** Kolmogrov's Mapping Neural Network Existence Theorem. *Proc. First IEEE Int. Joint Conf. Neural Networks*, San Diego, CA, June 21–24, Vol. 2, pp. 19–32.
- Hecht-Nielsen, R. (1989).** Theory of the Back-propagation Neural Network. In *Proceedings of the International Joint Conference on Neural Networks*, Washington, DC, Vol. 1, pp. 593–605.
- Hecht-Nielsen, R. (1990).** *Neurocomputing*, Addison-Wesely Publishing Company, Reading, MA.
- Hill, R. (1950).** *The Mathematical Theory of Plasticity*. Oxford University Press. London.
- Hjiaj, M., Lyamin, A. V. and Sloan, S. W. (2005).** Numerical Limit Analysis Solutions for the Bearing Capacity Factor N_{\square} . *International Journal of Solids and Structures*, Vol. 42, No. 5–6, pp. 1681–1704.
- Hornik, K. Stinchcombe, M. and White, H. (1989).** Multilayer Feedforward Networks are Universal Approximators. *Neural Networks*, Vol. 2, pp. 359–366.

- Houlsby, G. T., Milligan, G. W. E., Jewell, R. A. and Burd, H. J. (1989).** A New Approach to the Design of Unpaved Roads – Part 1. *Ground Engineering*, Vol. 22, No. 3, pp. 25–29.
- Hu, Y. and Randolph M. F. (1998).** A Practical Numerical Approach for Large deformation problems in soil. *International Journal for Numerical Analytical Methods in Geomechanics*. Vol. 22, pp. 327–350.
- Hubick, K. T. (1992).** *Artificial Neural Networks in Australia*, Department of Industry, Technology and Commerce, Commonwealth of Australia, Canberra.
- Hush, D. R. and Horne, B. G. (1993).** Progress in supervised neural networks. *IEEE ASSP Magazine*, January, pp. 8–37.
- Hyndman, R. J. (1990).** *PEST – A Program for Time Series Analysis*, Statistical Consulting Centre, University of Melbourne, 53 p.
- Isaaks, E. H. and Srivastava, R. M. (1989).** *Applied Geostatistics*, Oxford University Press, New York, 561 p.
- Jacobs, R. A. (1988).** Incremental Rates of Convergence Through Learning Rate Adaptation. *Neural Networks*, Vol. 1, pp. 295–307.
- Jaksa, M. B. (1995).** *Influence of Spatial Variability on the Geotechnical Design Properties of a Stiff, Over-consolidated Clay*, PhD Thesis, The University of Adelaide, 469 p.
- Jaksa, M. B., Brooker, P. I. and Kaggwa, W. S. (1996).** Modelling the Spatial Variability of the Undrained Shear Strength of Clay Soils Using Geostatistics. *Proceedings of Fifth Int. Geostatistics Congress*, Wollongong, (Publ. 1997), Kluwer Academic Publishers, Dordrecht, pp. 1284–1295.

- Jaksa, M. B., Brooker, P. I. and Kaggwa, W. S. (1999).** Closure: Inaccuracies Associated with Estimating Random Measurement Errors, *J. Geotech. and Geoenv. Engrg.*, ASCE, Vol. 125, No. 1, P. 81.
- Kaggwa, W. S. (2000).** Determination of the Spatial Variability of Soil Design Parameters and Its Significance in Geotechnical Engineering Analyses. *Development in Theoretical Geomechanics*, Balkema, Rotterdam, pp. 681–703.
- Karnin, E. D. (1990).** A Simple Procedure for Pruning Back-Propagation Trained Neural Networks." *IEEE Transactions on Neural Networks*, Vol. 1, No. 2, pp. 239–242.
- Kavli, T. (1993).** ASMOD – An Algorithm for Adaptive Spline Modelling of Observation Data. *Int. J. Control*, Vol. 58, No. 4, pp. 947–967.
- Koiter, W. (1953).** Anniversary Volume on Applied Mechanics Dedicated to B. Biezeno, H. Stam, Haarlem, pp. 232–251.
- Kudrycki, T. P. (1988).** *Neural network implementation of a medical diagnosis expert system*, MS thesis, College of Engineering, University of Cincinnati.
- Kulatilake, P. H. S. W., and Um, J. (2003).** Spatial Variation of Cone Tip Resistance for the Clay Site at Texas A & M University". *International Journal of Geotechnical and Geological Engineering*, Vol. 21, No. 2, pp. 149-165.
- Lachtermacher, G. and Fuller, J. D. (1994).** Back-propagation in Hydrological Time Series Forecasting. *Stochastic and Statistical Methods in Hydrology and Environmental Engineering*, K. W. Hipel, A. I. McLeod, U. S. Panu, and V. P. Singh, (eds.), Kluwer Academic, Dordrecht, pp. 229–242.
- Lapedes, A. and Farber, R. (1988).** How neural networks work. *Neural Information Processing Systems*, American Institute of Physics, pp. 442–456.

- Lee, I. K., White, W. and Ingles, O. G. (1983).** *Geotechnical Engineering*, Pittman Publishing Inc, Massachusetts, USA.
- Lewis, J. P. (1987).** Generalized Stochastic Subdivision, *ACM Transaction on Graphics*, Vol. 6, No. 3, pp. 167–190.
- Li, K. S. and White, W (1987).** Probabilistic Characterisation of Soil Profiles. *Research Report No. 19*, Australian Defence Force Academy, University of New South Wales, Canberra, ACT, Australia.
- Lin, S. and Dong, B. (1998).** Modeling of Fuzzy Machine Learning and Fuzzy Neural Network in Structural Design. *Uncertainty Modeling and Analysis in Civil Engineering*, B. M. Ayyub, (ed.), CRC Press, New York, pp. 337–355.
- Love, J. P., Burd, H. J., Miligan, G. W. E. and Houlsby, G. T. (1987).** Analytical and Model Studies of Reinforcement of a Layer of Granular Fill on a Soft Clay Subgrade. *Canadian Geotechnical Journal*, Vol. 24, pp. 611–622.
- Lumb, P. (1966).** The Variability of Natural Soils. *Canadian Geotechnical Journal*, Vol. 3, No. 2, pp. 74–97.
- Lumb, P. (1970).** Safety Factors and the Probability Distribution of Soil Strength. *Canadian Geotechnical Journal*, Vol. 7, No. 3, pp. 225–242.
- Lyamin, A. V. and Sloan, S. W. (2002a).** Lower Bound Limit Analysis Using Non-linear Programming. *International Journal for Numerical Methods in Engineering*, Vol. 55, pp. 573–611.
- Lyamin, A. V. and Sloan, S. W. (2002b).** Upper Bound Limit Analysis Using Linear Finite Elements and Nonlinear Programming. *International Journal for Numerical and Analytical Methods in Geomechanics*, Vol. 26, pp. 181–216.
- Lyamin, A.V. and Sloan, S. W. (2003).** Mesh Generation for Lower Bound Limit Analysis. *Advances in Engineering Software*, Vol. 34, No. 6, pp. 321–338.

- Lysmer, J. (1970).** Limit Analysis of Plane Problems in Soil Mechanics. *Journal of the Soil Mechanics and Foundations Division, ASCE*, Vol. 96, SM4, pp. 1131–1334.
- Makrodimopoulos, A. and Martin, C.M. (2005).** Limit Analysis using Large-Scale SOCP Optimization. Proceeding of 13th National Conference of UK ACME, Sheffield, pp. 21–24.
- Maier, H. R. (1995).** *Use of artificial neural networks for modelling multivariate water quality time series*, PhD. thesis, University of Adelaide, Adelaide, SA, Australia.
- Maier, R. M. and Dandy, G. C. (1998).** The effect of internal parameters and geometry on the performance of back-propagation neural networks: An empirical study. *Environmental Modelling & Software*, Vol. 13, pp. 193–209.
- Maren, A., Harston, C. and Pap, R. (1990).** *Handbook of Neural Computing Applications*, Academic Press, San Diego, California.
- Masters, T. (1993).** *Practical Neural Network Recipes in C++*, Academic Press, San Diego, California, 493 p.
- Matheron, G. (1970).** *The Theory of Regionalized Variables and its Applications*. Les Cahiers du Centre de Morphologie Mathématique de Fontainebleau, No. 5, Paris, Ecole Nationale Supérieure des Mines.
- McCulloch, W. S., and Pitts, W. (1943).** A Logical Calculus of Ideas Imminent in Nervous Activity. *Bulletin and Mathematical Biophysics*, Vol. 5, pp. 115–133.
- Merifield, R. S. (2002).** *Numerical Modelling of Soil Anchors*, PhD Thesis, University of Newcastle, NSW, Australia.

- Merifield, R. S., Sloan S. W. and Yu, H. S. (1999).** Rigorous Solutions for the Bearing Capacity of Two Layered Clay Soils. *Géotechnique*, Vol. 49, No. 4, pp. 471–490.
- Meyerhof, G. G. (1963).** Some Recent Research on the Bearing Capacity of Foundations. *Canadian Geotechnical Journal*, Vol. 1, No.1, pp. 16–26.
- Meyerhof, G. G. (1965).** Shallow Foundations. *Journal of Soil Mechanics and Foundation Division*. ASCE. Vol. 91, SM2, pp. 21–31.
- Meyerhof, G. G. (1974).** Ultimate Bearing Capacity of Footing on Sand Layer Overlying Clay. *Canadian Geotechnical Journal*, Vol. 11, No. 2, pp. 224–229.
- Meyerhof, G. G. and Hanna, A. M. (1978).** Ultimate Bearing Capacity of Foundations on Layered Soils under Inclined Load. *Canadian Geotechnical Journal*, Vol. 15, pp. 565–572.
- Michalowski, R. L. and Shi, L. (1995).** Bearing Capacity of Footings over Two-Layer Foundation Soils. *Journal of Geotechnical Engineering*, ASCE, Vol. 121, No. 5, pp. 421–428.
- Miller, G. F., Todd, P. M. and Hedge, S. U. (1989).** Designing Neural Networks using Genetic Algorithms. *Proc. 3rd Int. Conf. on Genetic Algorithms*, Arlington, pp. 379–384.
- Moselhi, O., Hegazy, T. and Fazio, P. (1992).** Potential Applications of Neural Networks in Construction. *Can. J. Civil Eng.*, Vol. 19, pp. 521–529.
- Mukherjee, A. and Deshpande, J. M. (1997).** Closure on: Modeling Initial Design Process using Artificial Neural Networks. *J. Computing in Civil Eng.*, Vol. 11, No. 2, pp. 145–146.

- Nagtegaal, J. C., Parks, D. M. and Rice, J. R. (1974).** On Numerically Accurate Finite Element Solutions in the Fully Plastic Range. *Computer Methods in Applied Mechanics and Engineering*, Vol. 4, pp. 153–177.
- Najjar, Y. M., Ali, H. E. and Basheer, I. A. (1999).** On the Use of Neuronets for Simulating the Stress-strain Behavior of Soils. *Proc. 7th Int. Symposium on Numerical Models in Geomechanics*, Graz, Austria, Vol. VII, pp. 657–662.
- Najjar, Y. M., Basheer, I. A. and Naouss, W. A. (1996).** On the Identification of Compaction Characteristics by Neuronets. *J. Computers and Geotechnics*, Vol. 18, No. 3, pp. 167–187.
- Nawari, N. O., Liang, R. and Nusairat, J. (1999).** Artificial intelligence techniques for the design and analysis of deep foundations. *Electronic J. Geotech. Eng.*, ppr 9909.
- NeuralWare (1997).** *NeuralWorks Predict Release 2.1*, NeuralWare Inc., Pittsburgh.
- Okamura, M., Takemura, J. and Kimura, T. (1998).** Bearing Capacity Predictions of Sand Overlying Clay Based on Limit Equilibrium Methods. *Soils and Foundations*, Vol. 38, pp. 181–194.
- Paice, G. M., Griffith, D. V. and Fenton, G. A. (1996).** Finite Element Modelling of Settlements on Spatially Random Soil. *Journal of Geotechnical Engineering*, ASCE, Vol. 112, No. 1, pp. 777–779.
- Pastor, J. (1978).** Limit Analysis: Numerical Determination of Complete Statical Solutions: Application to the Vertical Cut. *Journal Méchanique Appliquée*, Vol. 2, pp. 167–196.
- Phoon, K. K., and Kulhawy, F. H. (1999a).** Characterization of Geotechnical Variability. *Canadian Geotechnical Journal*, Vol. 36, No. 4, pp. 612–624.

- Phoon, K. K., and Kulhawy, F. H. (1999b).** Evaluation of Geotechnical Property Variability. *Canadian Geotechnical Journal*, Vol. 36, No. 4, pp. 625–639.
- Popescu, R., Deodatis, G. and Nobahar, A. (2003).** Fragility Curves for Bearing Capacity of Heterogeneous Soils. *Proceeding 9th ICASP*, San Francisco, CA, USA, Vol. 2, pp. 1357–1364.
- Poulos, H. G., Carter J. P. and Small J. C. (2001).** Foundations and Retaining Structures – Research and Practice (Theme Lecture). *Proceeding the 15th International Conference on Soil Mechanics and Geotechnical Engineering*, Istanbul, Turkey, Vol. 4, pp. 2527–2606.
- Prager, W. and Hodge, P. G. (1951).** *Theory of Perfectly Plastic Solids*, Wiley, New York.
- Prandtl, L. (1921).** Über die Eindringungs-festigkeigt (Härte) plastischer Bautoffe und die Festigkeit von Schneiden. *Zeitschrift für Angewandte Mathematik und Mechanik*, Vol. 1, No.1, pp. 15–20.
- Priestley, M. (1981).** *Spectral Analysis and Time Series. I: Univariate Series*. Academic Press, New York.
- Ray, C. and Klindworth, K. K. (2000).** Neural Networks for Agrichemical Vulnerability Assessment of Rural Private Wells. *J. Hydrologic Eng.*, Vol. 5, No. 2, pp. 162–171.
- Reddy, A. S. and Srinivasan, R. J. (1967).** Bearing Capacity of Footings on Layered Clays. *J. Soil Mech. Found. Div.*, ASCE, Vol. 93, No. 2, pp. 83–99.
- Reissner, H. (1924).** Zum Erddruckproblem. *Proc. First Congr. Appl. Mech.*, In: Biezend, C.B., Burgers, J.M. (Eds.), pp. 295–311.
- Rezania, M. and Javadi, A. A. (2007).** A New Genetic Programming Model for Predicting Settlement of Shallow Foundations. *Canadian Geotechnical Journal*, Vol. 44, pp. 1462–1473.

- Righetti, G. and Harrop-Williams, K. (1988).** Finite Element Analysis of Random Soil Media. *Journal of Geotechnical Engineering Division*, ASCE, Vol. 114, No. GT1, pp. 59–75.
- Ripley, B. D. (1996).** *Pattern recognition and neural networks*, Cambridge University Press, Cambridge.
- Rogers, L. L. and Dowla, F. U. (1994).** Optimization of Groundwater Remediation Using Artificial Neural Networks with Parallel Solute Transport Modeling. *Water Resources Research*, Vol. 30, No. 2, pp. 457–481.
- Rojas, R. (1996).** *Neural Networks: A Systematic Introduction*, Springer-Verlag, Berlin.
- Rumelhart, D. E., Hinton, G. E. and Williams, R. J. (1986).** Learning Internal Representation by Error Propagation. *Parallel Distributed Processing*, D. E. Rumelhart and J. L. McClelland, (eds.), MIT Press, Cambridge, pp. 318–362.
- Salchenberger, L. M., Cinar, E. M. and Lash, N. A. (1992).** Neural Networks: A New Tool for Predicting Thrift Failures. *Decision Science*, Vol. 23, pp. 899–916.
- Sarle, W. S. (1994a).** Neural Network Implementation in SAS Software. *Proc. Nineteenth Annual SAS Users Group Int. Conf.*, Cary, NC: SAS Institute, pp. 1–28.
- Sarle, W. S. (1994b).** Neural Networks and Statistical Models. *Proc. Nineteenth Annual SAS Users Group International Conference*, Cary, NC: SAS Institute, pp. 1538–1550.
- Schmertmann, J. H., Hartman, J. P. and Brown, P. B. (1978).** Improved Strain Influence Factor Diagrams. *Journal of Geotechnical and Geoenvironmental Engineering*, Vol. 104, No. 8, pp. 1131–1135.

- Schultze, E. and Sherif, G. (1973).** Prediction of Settlements from Evaluated Settlement Observations for Sand. *Proc. 8th Int. Conf. Soil Mechanics and Foundation Engineering*, Vol. 1, No. 3, pp. 225–230.
- Shahin, M. A. (2003).** Use of Artificial Neural Networks for Predicting Settlement of Shallow Foundations on Cohesionless Soils. PhD Thesis, The University of Adelaide, SA, Australia.
- Shahin, M. A., Jaksa M. B. and Maier H. R. (2001).** Artificial Neural Network Application in Geotechnical Engineering. *Australian Geomechanics*, Vol. 36, No. 1, pp. 49–62.
- Shahin, M.A., Jaksa, M. B. and Maier, H. R. (2002).** Artificial Neural Network-Based Settlement Prediction Formula for Shallow Foundations on Granular Soils. *Australian Geomechanics*, Vol. 37, No. 4, pp. 45–52.
- Shahin, M. A., Maier, H. R. and Jaksa, M. B. (2004).** Data Division for Developing Neural Networks Applied to Geotechnical Engineering. *J. Computing in Civil Engrg.*, ASCE, Vol. 18, No. 2, pp. 105–114.
- Shanley, F. R. (1947).** Inelastic Column Theory, *Journal of the Aeronautical Sciences*, Vol. 14, No. 5, pp. 261–267
- Shiau, J. S., Lyamin, A. V. and Sloan, S. W. (2003).** Bearing Capacity of a Sand Layer on Clay by Finite Limit Analysis. *Canadian Geotechnical Journal*, Vol. 40, pp. 900–915.
- Sietsma, J. and Dow, R. J. F. (1988).** Neural Net Pruning – Why and How. *Proc. IEEE Int. Conf. Neural Networks*, San Diego, CA, pp. 325–333.
- Sloan, S. W. (1981).** *Numerical Analysis of Incompressible and Plastic Solids Using Finite Elements*, PhD Thesis, University of Cambridge, Cambridge, U.K.

- Sloan, S. W. (1988).** Lower Bound Limit Analysis Using Finite Elements and Linear Programming. *International Journal for Numerical and Analytical Methods in Geomechanics*, Vol. 12, pp. 61–67.
- Sloan, S. W. (1989).** Upper Bound Limit Analysis Using Finite Elements and Linear Programming. *International Journal for Numerical and Analytical Methods in Geomechanics*, Vol. 13, pp. 263–282
- Sloan, S. W. and Kleeman, P. W. (1995).** Upper Bound Limit Analysis Using Discontinuous Velocity Fields. *Computer Methods in Applied Mechanics and Engineering*, Vol. 127, pp. 293–314.
- Sloan, S. W. and Randolph, M. F. (1982).** Numerical Prediction of Collapse Load Using Finite Element Methods. *International Journal for Numerical and Analytical Methods in Geomechanics*, Vol. 6, pp. 47–76.
- Smith, M. (1993).** *Neural Networks for Statistical Modeling*, Van Nostrand Reinhold, New York.
- Smith, I. M. and Griffiths, D. V. (1998).** *Programming the Finite Element Method*, 3rd Edition. John Wiley & Son, England, 546p.
- Spry, M. J., Kulhawy, F. H. and Grigoriu, M. D. (1988).** Reliability-Based Foundation Design For Transmission Line Structures, Volume 1: Geotechnical Site Characterization Strategy, *Report E1-5007, Volume 1*, Electrical Power Research Institute, Palo Alto, California.
- Stone, M. (1974).** Cross-Validatory Choice and Assessment of Statistical Predictions. *J. Royal Statistical Society*, Vol. B 36, pp. 111–147.
- Symonds, P. S. and Neal, B. G. (1951).** Recent Progress in the Plastic Method Structural Analysis. *J. Franklin Institute*, Vol. 5, pp. 387–407.

- Tani, K. and Craig, W. H. (1995).** Bearing Capacity of Circular Foundations on Soft Clay of Strength Increasing with Depth. *Soils and Foundations*, Vol. 35, pp. 21–35.
- Terzaghi, K. (1943).** *Theoretical Soil Mechanics*, John Wiley & Sons, New York.
- Tokar, S. A. and Johnson, P. A. (1999).** Rainfall-Runoff Modeling Using Artificial Neural Networks. *J. Hydrologic Eng.*, Vol. 4, No. 3, pp. 232–239.
- Touretzky, D. S. and Pomerleau, D. A. (1989).** What's Hidden in the Hidden Layers? *Byte*, August, pp. 227–233.
- Twomey, J. M. and Smith, A. E. (1997).** Validation and Verification. *Artificial neural networks for civil engineers: Fundamentals and applications*, N. Kartam, I. Flood, and J. H. Garrett, (eds.), ASCE, New York, pp. 44–64.
- Ural, D. N. and Saka, H. (1998).** Liquefaction Assessment by Neural Networks. *Elect. J. Geotech. Eng.*, <http://www.ejge.com/Ppr9803/Ppr9803.htm>. Accessed Online.
- Vanmarcke, E. H. (1977a).** Probabilistic Modeling of Soil Profiles. *Journal of Geotechnical Engineering Division*, ASCE, Vol. 103, No. 11, pp. 1227–1246.
- Vanmarcke, E. H. (1977b).** Reliability of Earth Slope. *Journal of Geotechnical Engineering Division*, ASCE, Vol. 103, No. 11, pp. 1247–1265.
- Vanmarcke, E. H. (1978).** Probabilistic Characterization of Soil Profiles. In *Site Characterization and Exploration*, Proc. Specialty Workshop, ASCE, Northwestern Uni., Evanston, Ill., pp. 199–216.
- Vanmarcke, E. H. (1983).** *Random Fields: Analysis and Synthesis*, The MIT Press, Cambridge, Massachusetts, USA.
- Vesić, A. S. (1973).** Analysis of Ultimate Loads of Shallow Foundation. *J. Soil Mech. Found. Div.*, ASCE, Vol. 99, No. SM1, pp. 45–73.

- Vesić, A. S. (1975).** Bearing Capacity of Shallow Foundations. *Foundation Engineering Handbook*, Van Nostrand Reinhold, pp. 121–147.
- Vitela, J. E. and Reifman, J. (1997).** Premature Saturation in Back-propagation Networks – Mechanism and Necessary Conditions. *Neural Networks*, Vol. 10, No. 4, pp. 721–735.
- Wasserman, P. D. (1989).** *Neural computing theory and practice*, Van Nostrand Reinhold, New York.
- Weiss, S. and Kulikowski, C. A. (1991).** *Computer Systems that Learn: Classification and Prediction Methods from Statistics, Neural Nets, Machine Learning, and Expert Systems*, Morgan Kaufmann Publishers, San Francisco, CA.
- White, H. (1989).** Learning in artificial neural networks: A statistical perspective. *Neural Computation*, Vol. 1, pp. 425–464.
- Wingle, W. L. (1997).** *Evaluating Subsurface Uncertainty Using Modified Geostatistic Techniques*, PhD Dissertation #T-4595, Department of Geology and Geological Engineering, Colorado School of Mines, 202p.
- Yu, X. H. (1992).** Can Back-Propagation Error Surface not Have Local Minima. *IEEE Transaction on Neural Networks*, Vol. 3, pp. 1019–1021.
- Zhang, M. M. (1997).** *Neural Network Material Models Determined from Structural Tests*, PhD. thesis, University of Illinois at Urbana-Champaign, Urbana, Illinois.
- Zienkiewicz, O. C. (1991).** *The Finite Element Method*, Fourth Edition, McGraw-Hill, New York.
- Zurada, J. M. (1992).** *Introduction to Artificial Neural Systems*. West Publishing Company, St. Paul.

APPENDIX A

Table A.1 Lower bound estimation for $c_{u1} / c_{u2} \leq 1.0$.

c_1/c_2 H/B	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
0.1	8.046	8.045	8.043	8.045	7.962	7.320	6.659	6.074	5.529	5.052
0.2	6.287	6.285	6.287	6.288	6.288	6.287	6.219	5.859	5.452	5.052
0.3	5.646	5.645	5.647	5.645	5.646	5.647	5.646	5.630	5.371	5.052
0.4	5.339	5.340	5.340	5.340	5.336	5.337	5.337	5.338	5.288	5.052
0.5	5.172	5.170	5.173	5.173	5.172	5.172	5.171	5.173	5.173	5.052
0.6	5.083	5.085	5.083	5.085	5.084	5.085	5.083	5.084	5.084	5.052
0.7	5.055	5.055	5.057	5.055	5.052	5.055	5.054	5.054	5.053	5.052
0.8	5.052	5.050	5.053	5.053	5.054	5.053	5.053	5.054	5.053	5.052
0.9	5.053	5.055	5.053	5.053	5.052	5.050	5.053	5.053	5.053	5.052
1.0	5.054	5.055	5.053	5.050	5.054	5.053	5.053	5.053	5.053	5.052
1.1	5.052	5.055	5.053	5.053	5.054	5.053	5.053	5.053	5.053	5.052
1.2	5.053	5.055	5.050	5.053	5.054	5.052	5.053	5.054	5.053	5.052
1.3	5.053	5.055	5.050	5.053	5.054	5.052	5.053	5.054	5.053	5.052
1.4	5.053	5.055	5.050	5.053	5.052	5.052	5.053	5.053	5.053	5.052
1.5	5.052	5.050	5.053	5.053	5.052	5.052	5.053	5.053	5.053	5.052
1.6	5.052	5.055	5.050	5.053	5.052	5.052	5.053	5.053	5.053	5.052
1.7	5.052	5.055	5.053	5.050	5.052	5.052	5.053	5.053	5.053	5.052
1.8	5.053	5.055	5.053	5.053	5.052	5.053	5.051	5.053	5.053	5.052
1.9	5.052	5.055	5.053	5.053	5.052	5.053	5.053	5.054	5.053	5.052
2.0	5.053	5.055	5.053	5.053	5.052	5.053	5.053	5.053	5.053	5.052
2.1	5.053	5.055	5.053	5.053	5.052	5.053	5.053	5.054	5.053	5.052
2.2	5.053	5.055	5.053	5.053	5.052	5.053	5.053	5.053	5.053	5.052
2.3	5.053	5.055	5.053	5.053	5.052	5.052	5.053	5.053	5.053	5.052
2.4	5.053	5.055	5.053	5.053	5.054	5.053	5.053	5.053	5.053	5.052
2.5	5.053	5.055	5.053	5.053	5.052	5.053	5.053	5.053	5.053	5.052
2.6	5.053	5.050	5.053	5.050	5.052	5.053	5.053	5.053	5.053	5.052
2.7	5.053	5.055	5.053	5.053	5.052	5.053	5.053	5.054	5.053	5.052
2.8	5.053	5.050	5.053	5.053	5.052	5.053	5.053	5.054	5.053	5.052
2.9	5.053	5.055	5.053	5.053	5.052	5.053	5.053	5.054	5.053	5.052
3.0	5.053	5.055	5.053	5.053	5.052	5.053	5.053	5.054	5.053	5.052

Table A.2 Upper bound estimation for $c_{u1} / c_{u2} < 1.0$.

Table A.3 Lower bound estimation for $10.0 \geq c_{u1} / c_{u2} \geq 1.0$.

c_1/c_2 H/B	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0	10.0
0.1	5.052	2.766	1.967	1.552	1.293	1.111	0.975	0.871	0.788	0.722
0.2	5.052	3.000	2.230	1.812	1.544	1.353	1.210	1.101	1.013	0.941
0.3	5.052	3.232	2.480	2.059	1.780	1.585	1.439	1.321	1.226	1.147
0.4	5.052	3.447	2.723	2.303	2.014	1.808	1.653	1.531	1.430	1.347
0.5	5.052	3.655	2.955	2.535	2.246	2.035	1.873	1.745	1.640	1.553
0.6	5.052	3.858	3.180	2.765	2.472	2.260	2.094	1.963	1.856	1.768
0.7	5.052	4.052	3.403	2.988	2.698	2.485	2.323	2.193	2.088	2.001
0.8	5.052	4.241	3.620	3.213	2.926	2.718	2.560	2.435	2.334	2.249
0.9	5.052	4.426	3.833	3.435	3.160	2.958	2.806	2.684	2.584	2.503
1.0	5.052	4.597	4.040	3.658	3.396	3.203	3.054	2.938	2.843	2.763
1.1	5.052	4.756	4.237	3.878	3.630	3.448	3.310	3.198	3.107	3.031
1.2	5.052	4.898	4.417	4.085	3.862	3.693	3.561	3.459	3.372	3.299
1.3	5.052	5.030	4.583	4.278	4.074	3.923	3.807	3.714	3.632	3.568
1.4	5.052	5.055	4.737	4.455	4.268	4.130	4.027	3.944	3.874	3.818
1.5	5.052	5.050	4.880	4.618	4.442	4.317	4.221	4.145	4.084	4.034
1.6	5.052	5.050	5.013	4.768	4.604	4.487	4.394	4.325	4.269	4.221
1.7	5.052	5.055	5.050	4.913	4.756	4.645	4.559	4.491	4.440	4.394
1.8	5.052	5.055	5.053	5.043	4.900	4.795	4.713	4.650	4.597	4.553
1.9	5.052	5.055	5.053	5.053	5.034	4.935	4.859	4.798	4.748	4.706
2.0	5.052	5.055	5.053	5.053	5.052	5.053	4.996	4.936	4.890	4.850
2.1	5.052	5.055	5.053	5.053	5.052	5.052	5.051	5.054	5.023	4.986
2.2	5.052	5.055	5.053	5.053	5.052	5.053	5.053	5.054	5.052	5.053
2.3	5.052	5.055	5.053	5.053	5.054	5.053	5.053	5.054	5.052	5.052
2.4	5.052	5.055	5.053	5.053	5.054	5.053	5.053	5.054	5.053	5.053
2.5	5.052	5.055	5.053	5.053	5.054	5.053	5.053	5.054	5.053	5.052
2.6	5.052	5.055	5.053	5.053	5.052	5.053	5.053	5.054	5.053	5.053
2.7	5.052	5.055	5.053	5.053	5.054	5.053	5.053	5.054	5.053	5.053
2.8	5.052	5.055	5.053	5.053	5.052	5.053	5.053	5.054	5.053	5.052
2.9	5.052	5.055	5.053	5.053	5.052	5.052	5.053	5.054	5.052	5.052
3.0	5.052	5.055	5.053	5.053	5.052	5.053	5.053	5.054	5.052	5.052

Table A.4 Upper bound estimation for $10.0 \geq c_{u1} / c_{u2} \geq 1.0$.

c_1/c_2 H/B	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0	10.0
0.1	5.204	2.877	2.060	1.636	1.372	1.189	1.053	0.950	0.868	0.799
0.2	5.204	3.112	2.328	1.903	1.627	1.433	1.288	1.174	1.082	1.005
0.3	5.204	3.342	2.581	2.151	1.866	1.664	1.512	1.391	1.293	1.211
0.4	5.204	3.558	2.820	2.390	2.101	1.893	1.732	1.603	1.497	1.409
0.5	5.204	3.767	3.052	2.624	2.329	2.115	1.950	1.816	1.706	1.614
0.6	5.204	3.967	3.278	2.851	2.555	2.337	2.170	2.038	1.929	1.837
0.7	5.204	4.162	3.498	3.076	2.780	2.566	2.402	2.271	2.164	2.075
0.8	5.204	4.352	3.714	3.300	3.013	2.803	2.644	2.517	2.412	2.325
0.9	5.204	4.535	3.925	3.524	3.248	3.047	2.892	2.770	2.671	2.587
1.0	5.204	4.707	4.132	3.748	3.485	3.292	3.145	3.027	2.932	2.853
1.1	5.204	4.866	4.329	3.969	3.722	3.539	3.398	3.287	3.197	3.122
1.2	5.204	5.012	4.516	4.183	3.954	3.784	3.653	3.547	3.461	3.391
1.3	5.204	5.145	4.688	4.384	4.175	4.021	3.900	3.803	3.724	3.657
1.4	5.204	5.204	4.847	4.568	4.379	4.240	4.132	4.045	3.974	3.913
1.5	5.204	5.204	4.995	4.736	4.562	4.435	4.338	4.261	4.197	4.144
1.6	5.204	5.204	5.133	4.893	4.730	4.611	4.521	4.449	4.390	4.342
1.7	5.204	5.204	5.204	5.039	4.886	4.774	4.688	4.620	4.565	4.518
1.8	5.204	5.204	5.204	5.175	5.032	4.927	4.845	4.780	4.727	4.683
1.9	5.204	5.204	5.204	5.204	5.170	5.070	4.993	4.931	4.880	4.838
2.0	5.204	5.204	5.204	5.204	5.204	5.199	5.132	5.073	5.024	4.984
2.1	5.204	5.204	5.204	5.204	5.204	5.204	5.204	5.200	5.161	5.122
2.2	5.204	5.204	5.204	5.204	5.204	5.204	5.204	5.200	5.204	5.204
2.3	5.204	5.204	5.204	5.204	5.204	5.204	5.204	5.200	5.204	5.204
2.4	5.204	5.204	5.204	5.204	5.204	5.204	5.204	5.200	5.204	5.204
2.5	5.204	5.204	5.204	5.204	5.204	5.204	5.204	5.200	5.204	5.204
2.6	5.204	5.204	5.204	5.204	5.204	5.204	5.204	5.200	5.204	5.204
2.7	5.204	5.204	5.204	5.204	5.204	5.204	5.204	5.200	5.204	5.204
2.8	5.204	5.204	5.204	5.204	5.204	5.204	5.204	5.200	5.204	5.204
2.9	5.204	5.204	5.204	5.204	5.204	5.204	5.204	5.200	5.204	5.204
3.0	5.204	5.204	5.204	5.204	5.204	5.204	5.204	5.200	5.204	5.204

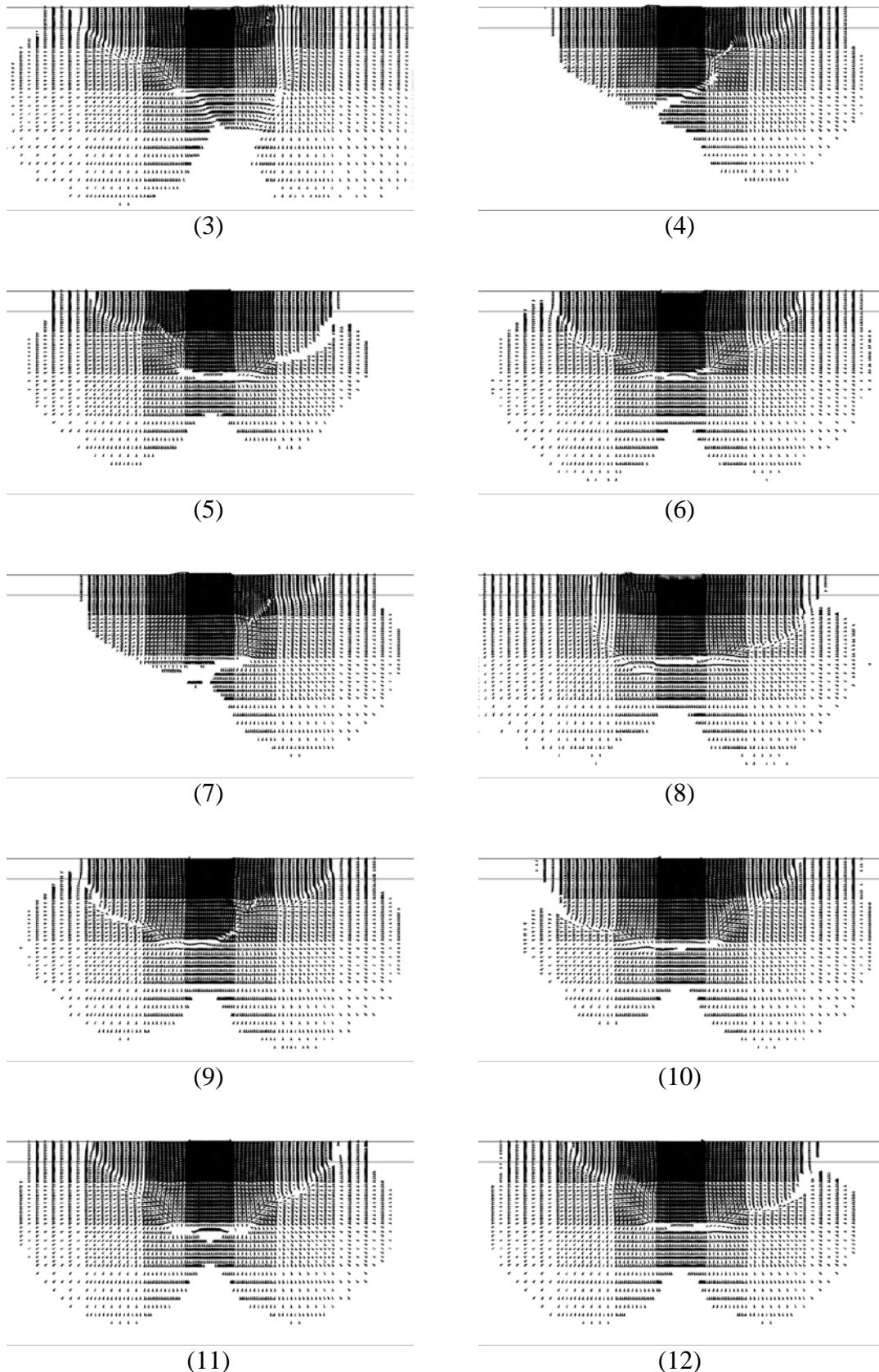


Figure A.1 Displacement vectors at near failure (two-layered spatially variable purely cohesive material).

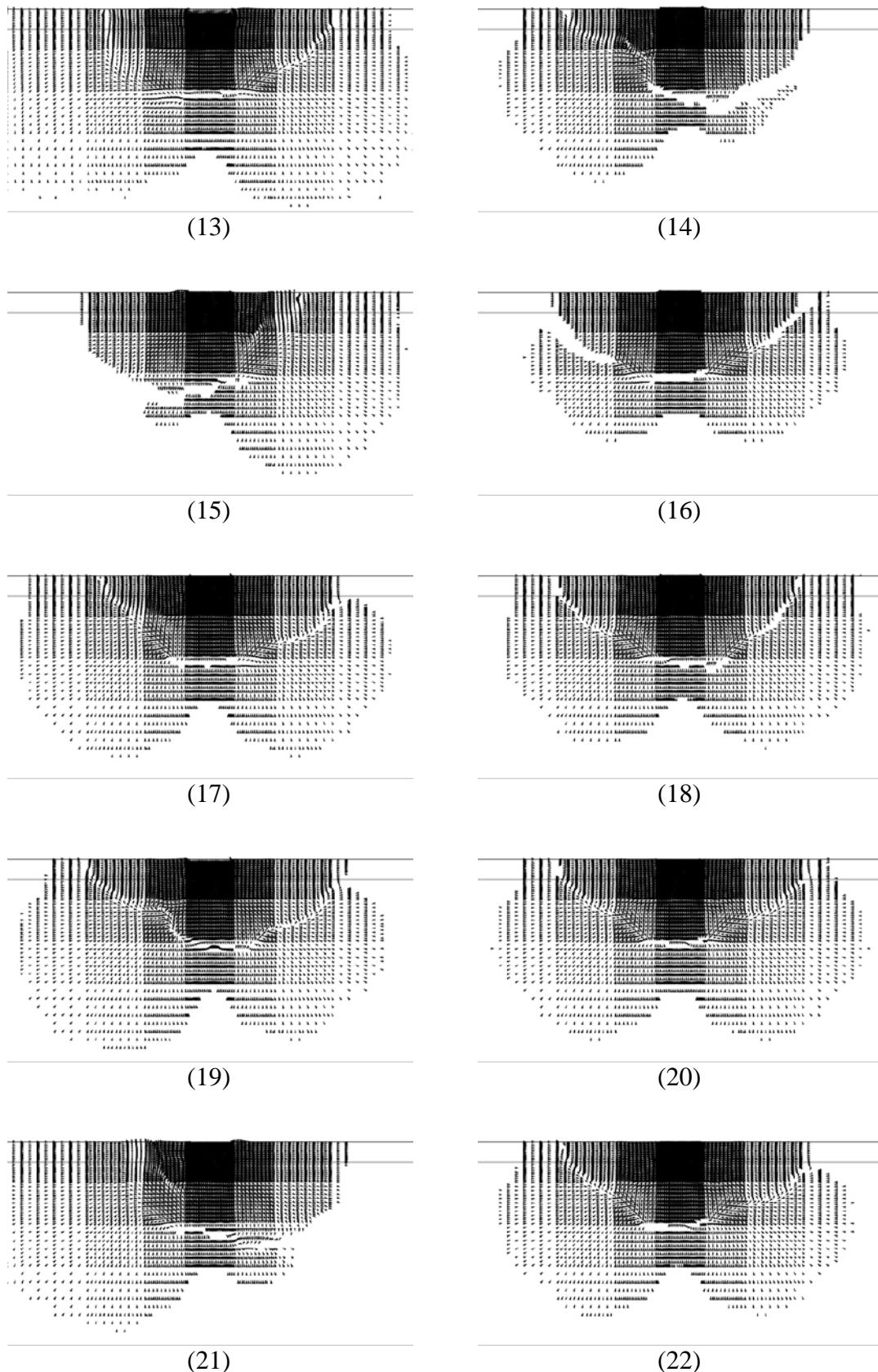


Figure A.1 Displacement vectors at near failure (two-layered spatially variable purely cohesive material). (*Continued*)

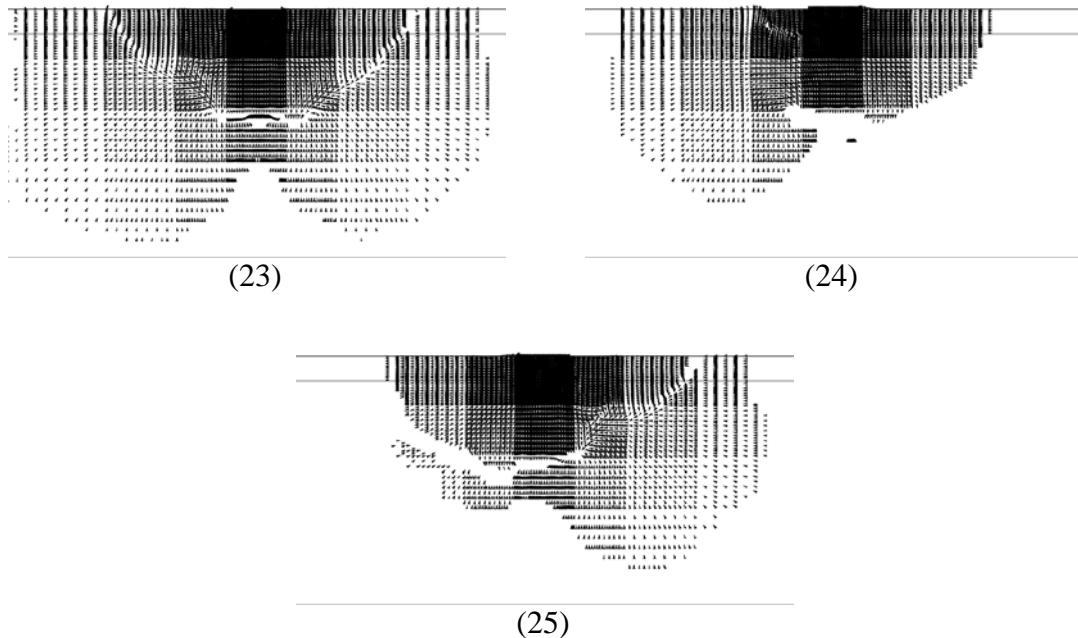


Figure A.1 Displacement vectors at near failure (two-layered spatially variable purely cohesive material). (*Continued*)

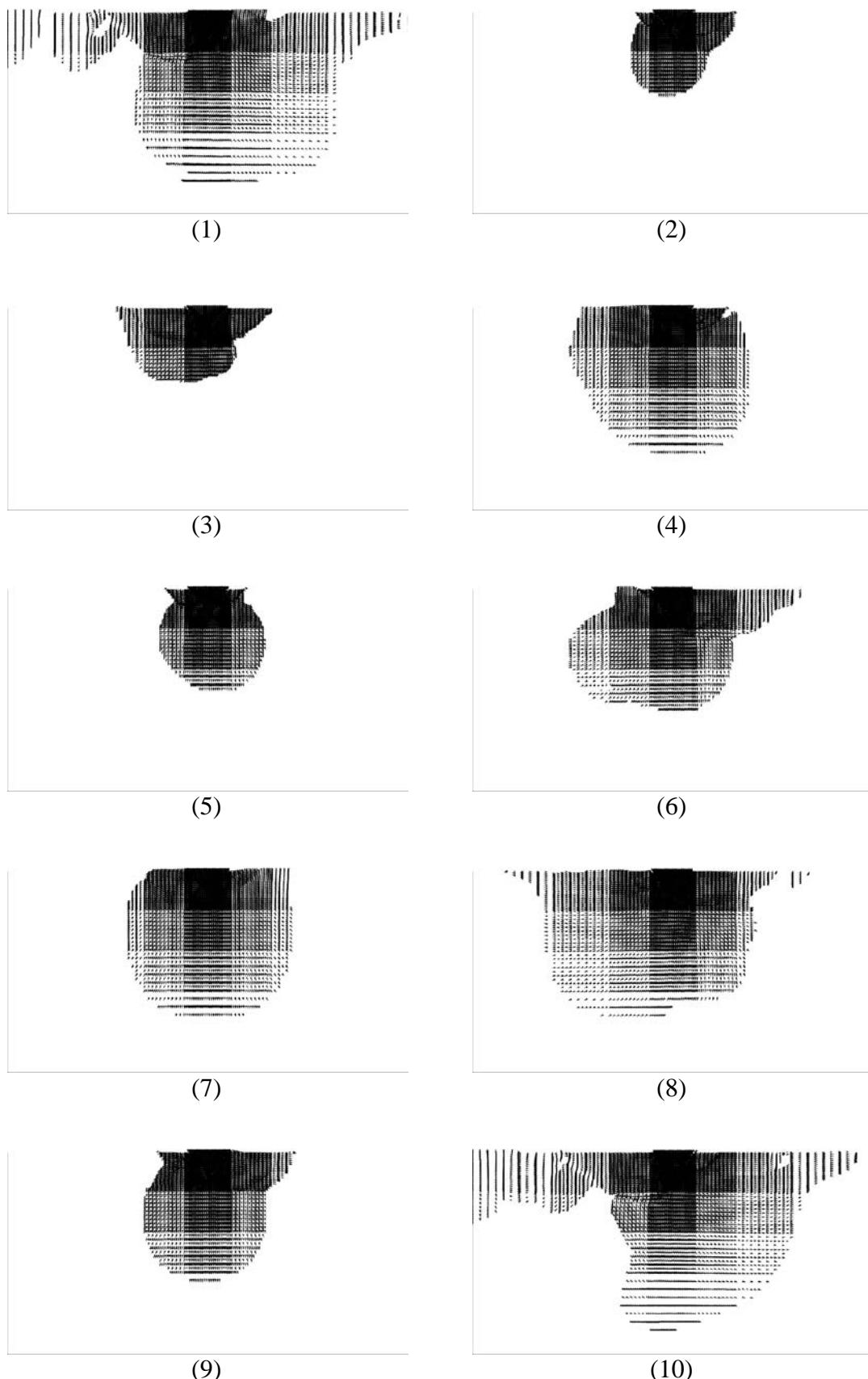


Figure A.2 Displacement vectors at near failure (single-layered spatially variable cohesive-frictional material).

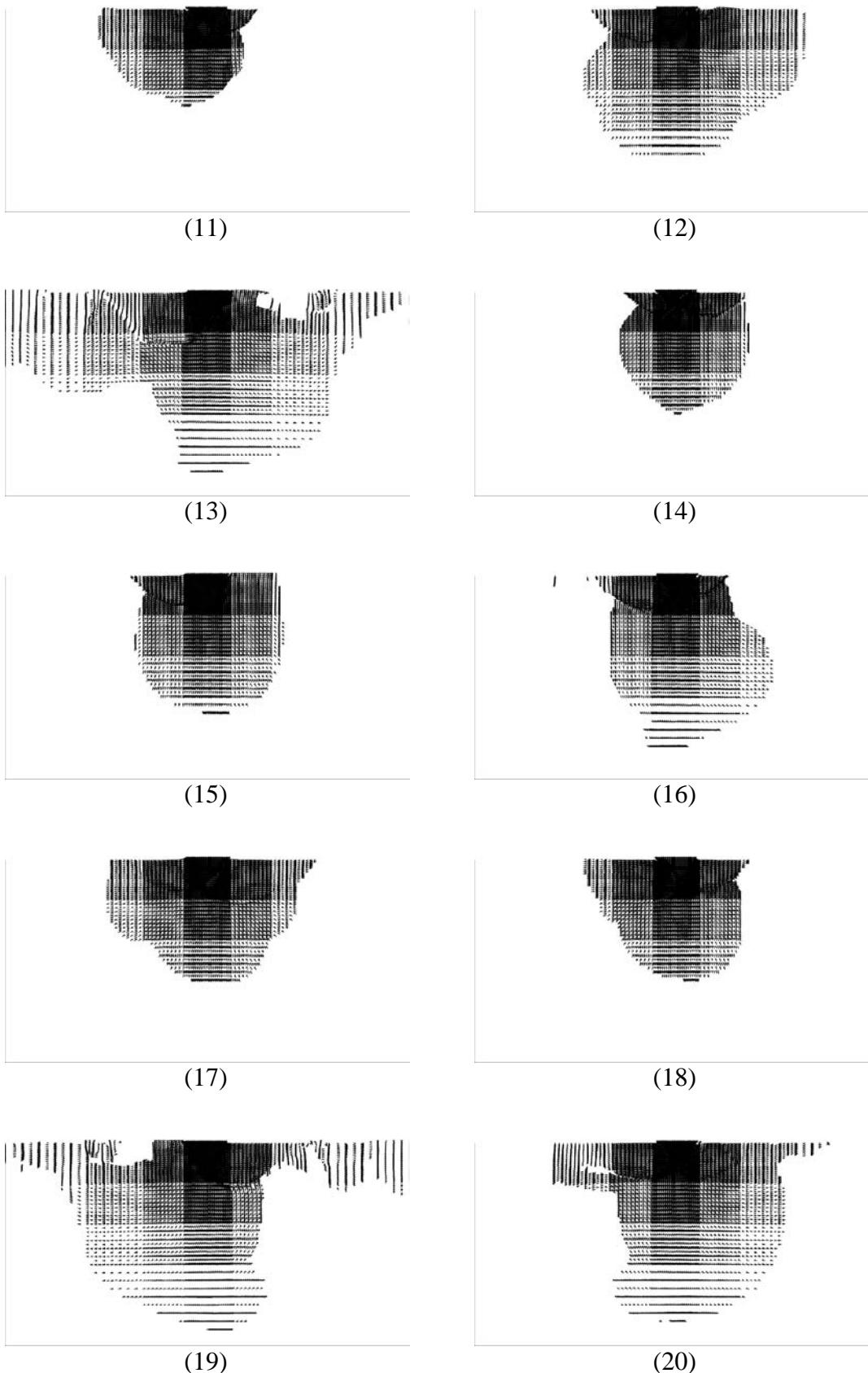


Figure A.2 Displacement vectors at near failure (single-layered spatially variable cohesive-frictional material). (*Continued*)

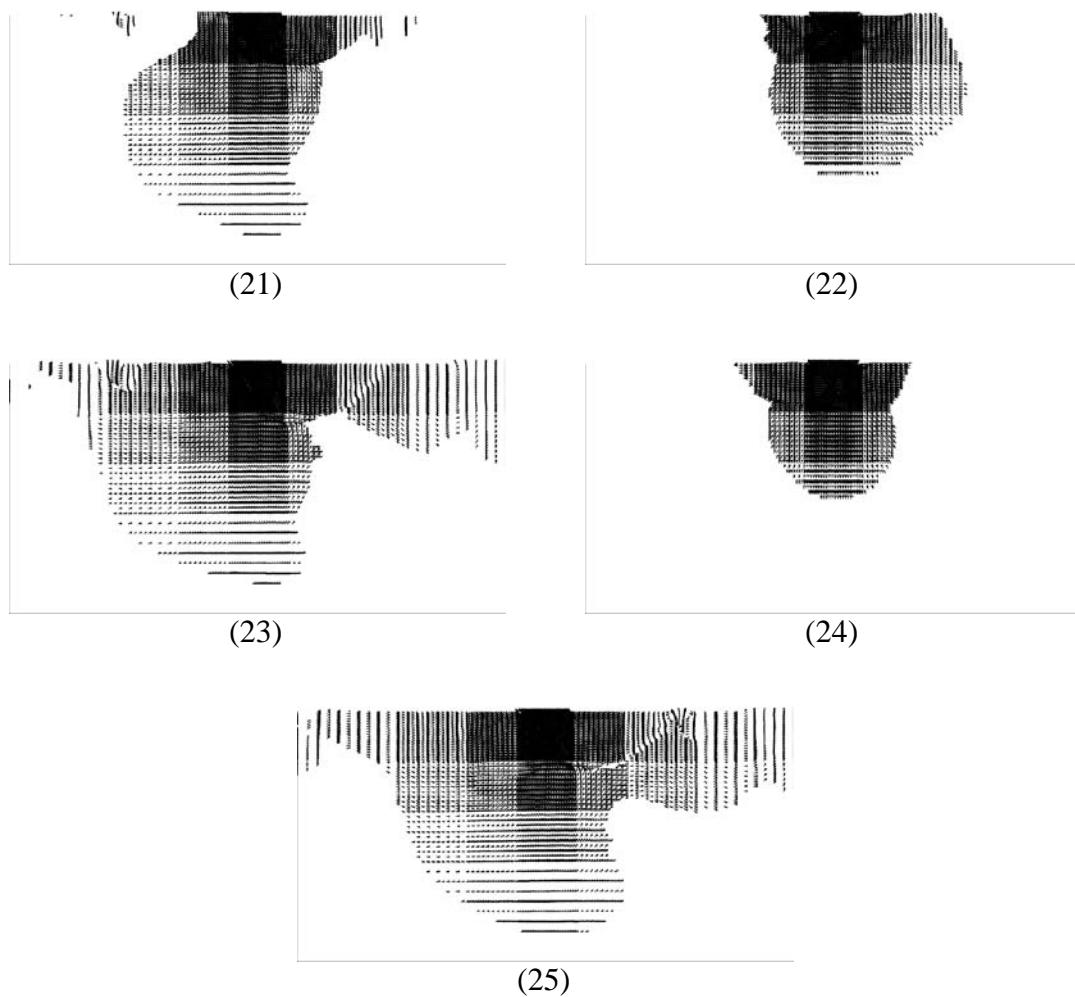


Figure A.2 Displacement vectors at near failure (single-layered spatially variable cohesive-frictional material). (*Continued*)

APPENDIX B

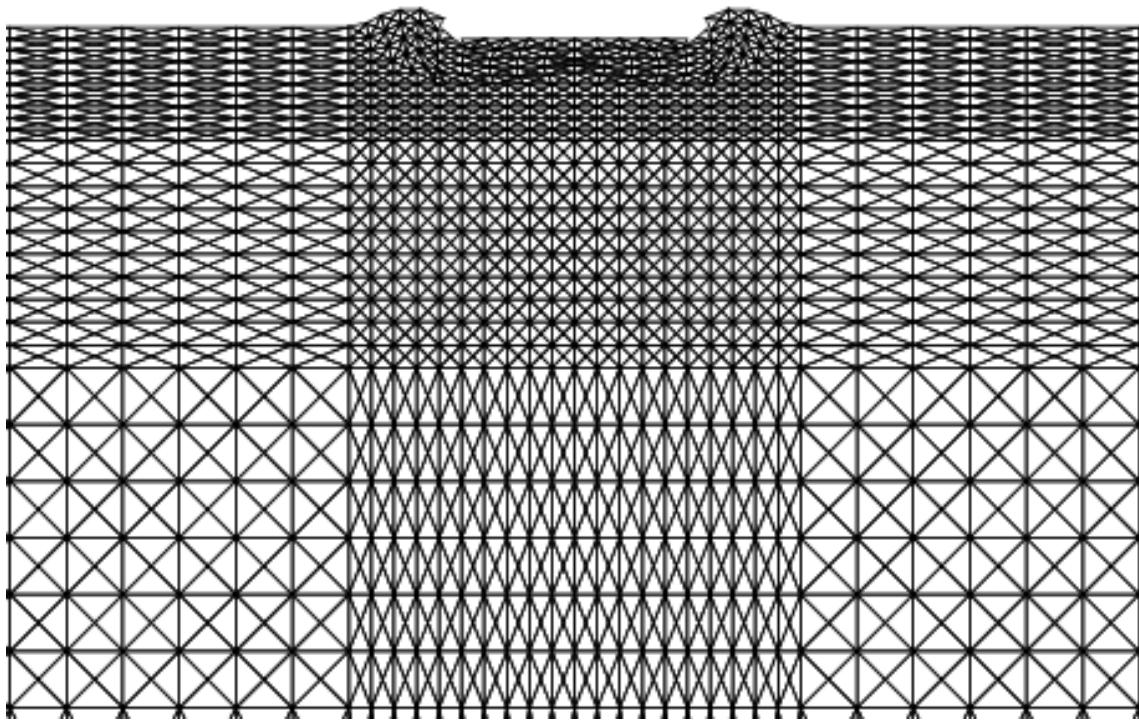


Figure B.1 Upper bound failure mechanism for COHESIVE_0.025_0.25 case (i.e.

$$c_{u1}/c_{u2} = 0.025, H/B = 0.25).$$

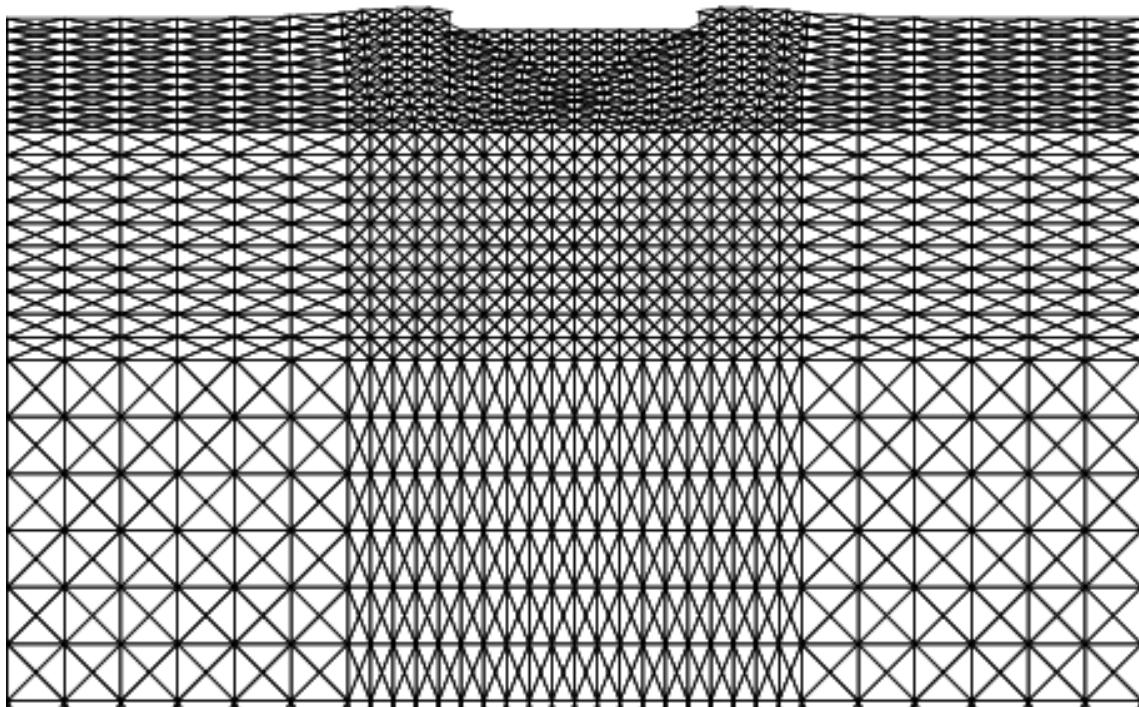


Figure B.2 Upper bound failure mechanism for COHESIVE_0.025_0.50 case (i.e.

$$c_{u1}/c_{u2} = 0.025, H/B = 0.5).$$

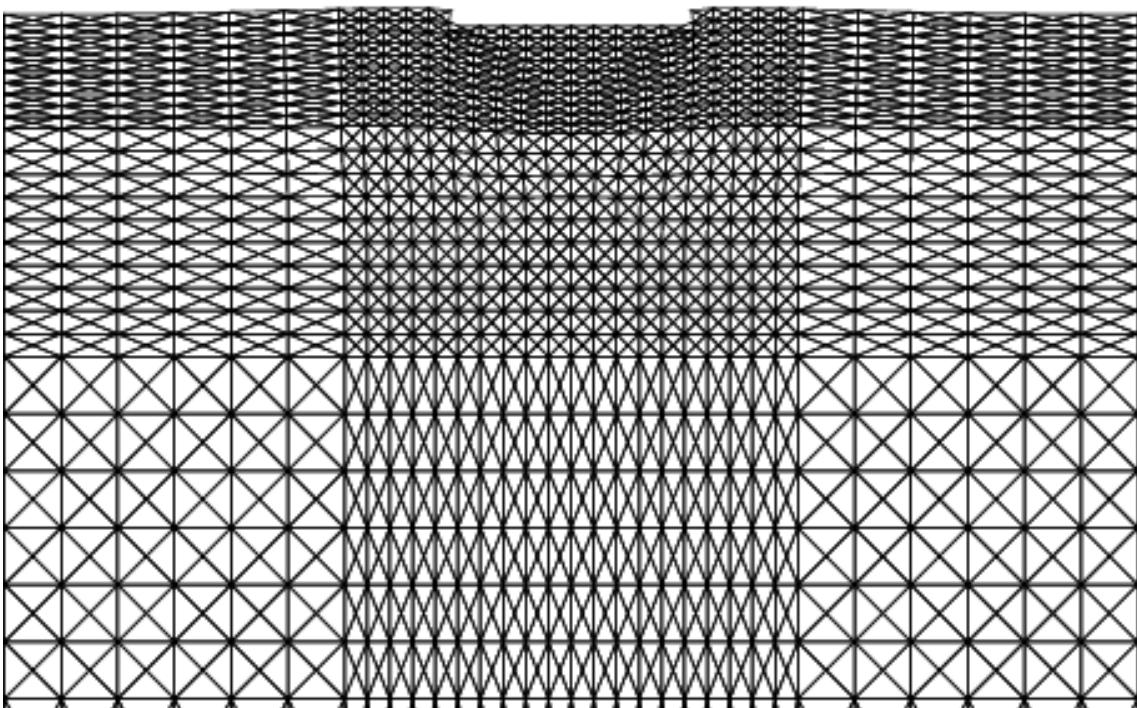


Figure B.3 Upper bound failure mechanism for COHESIVE_0.025_1.00 case (i.e.

$$c_{u1}/c_{u2} = 0.025, H/B = 1.0).$$

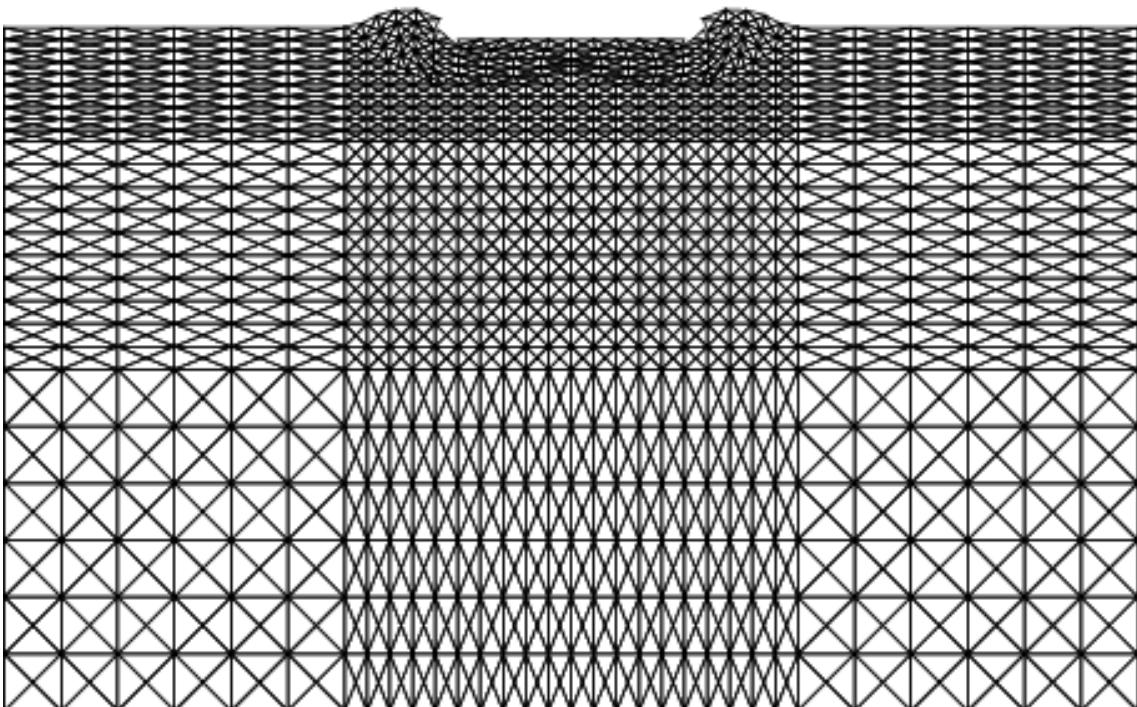


Figure B.4 Upper bound failure mechanism for COHESIVE_0.05_0.25 case (i.e.

$$c_{u1}/c_{u2} = 0.05, H/B = 0.25).$$

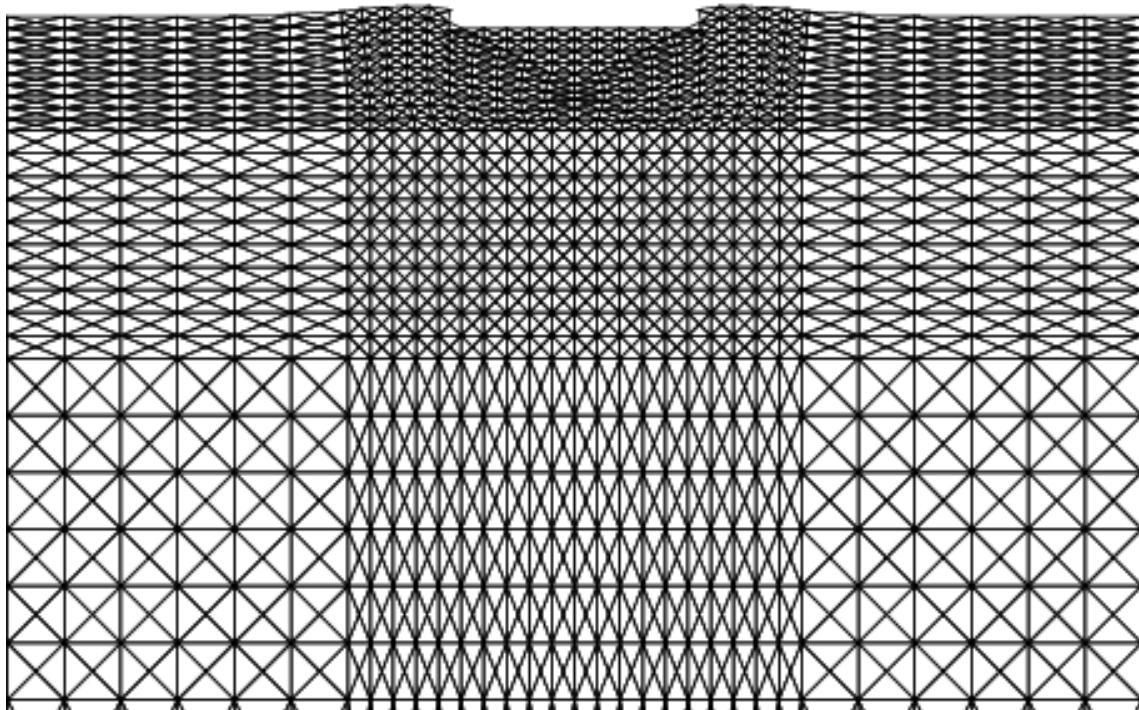


Figure B.5 Upper bound failure mechanism for COHESIVE_0.05_0.50 case (i.e. $c_{u1}/c_{u2} = 0.05$, $H/B = 0.5$).

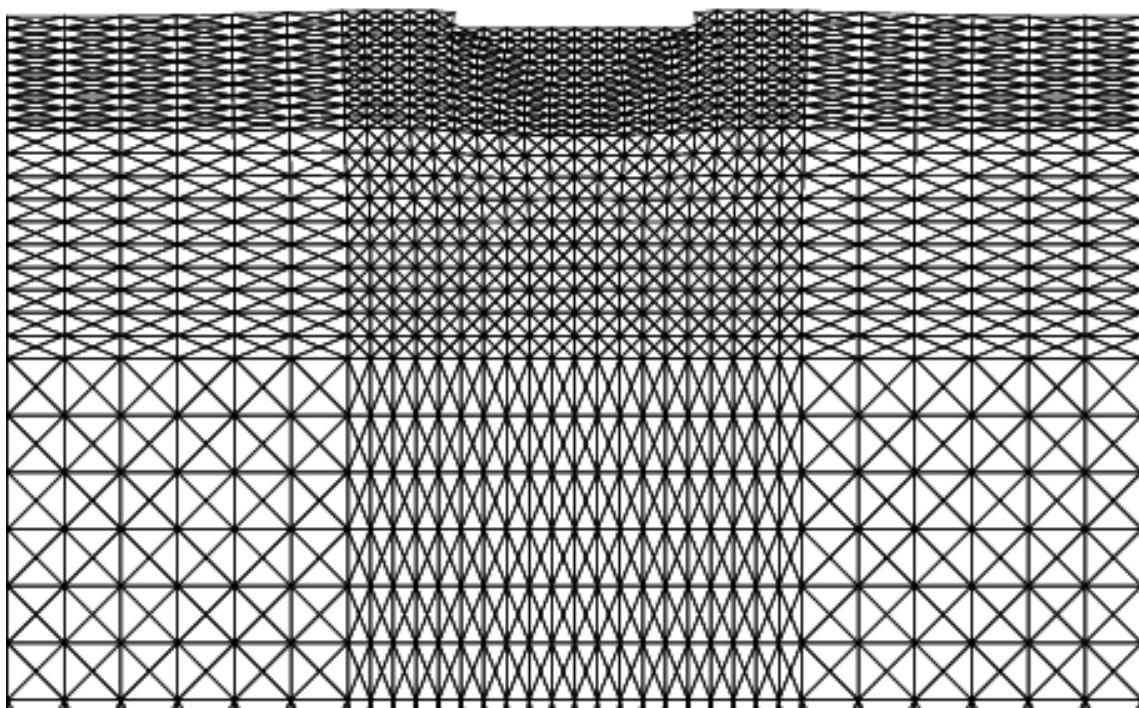


Figure B.6 Upper bound failure mechanism for COHESIVE_0.05_1.0 case (i.e. $c_{u1}/c_{u2} = 0.05$, $H/B = 1.0$).

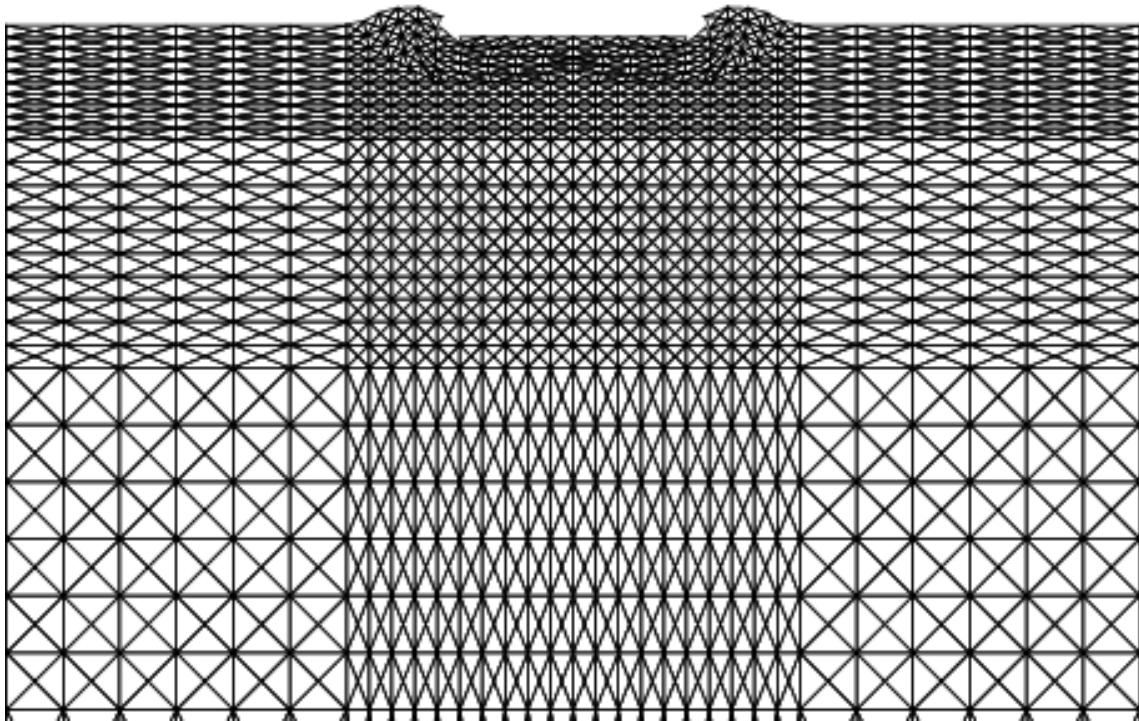


Figure B.7 Upper bound failure mechanism for COHESIVE_0.1_0.25 case (i.e. $c_{u1}/c_{u2} = 0.1$, $H/B = 0.25$).

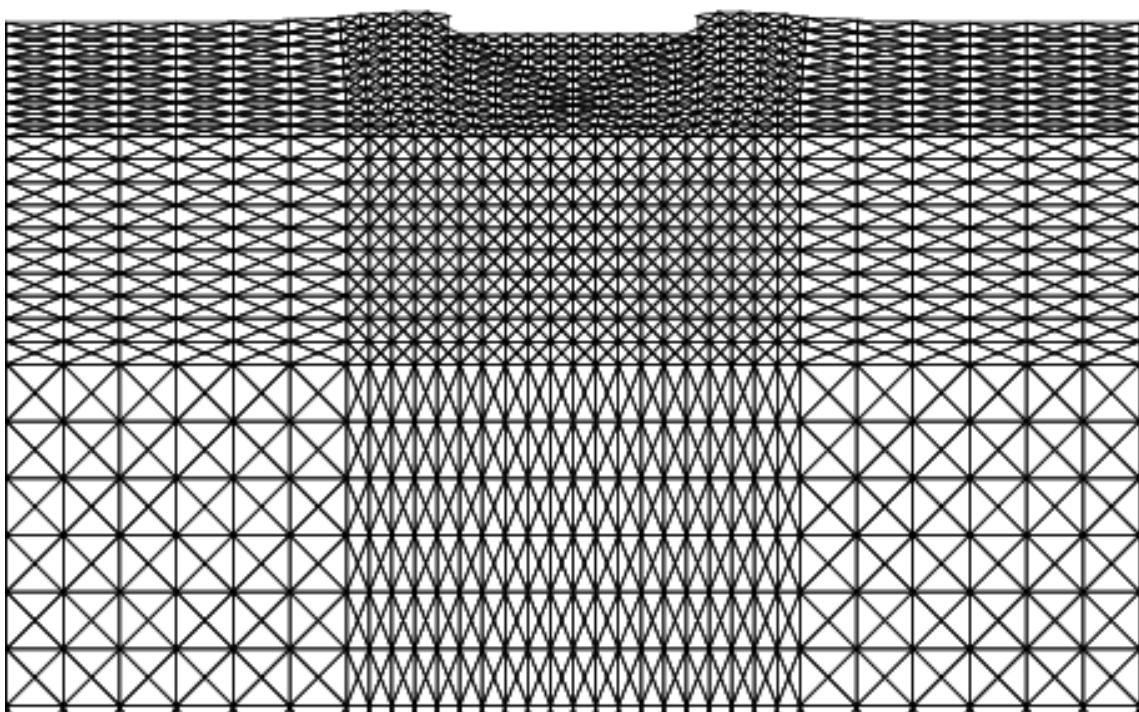


Figure B.8 Upper bound failure mechanism for COHESIVE_0.1_0.5 case (i.e. $c_{u1}/c_{u2} = 0.1$, $H/B = 0.5$).

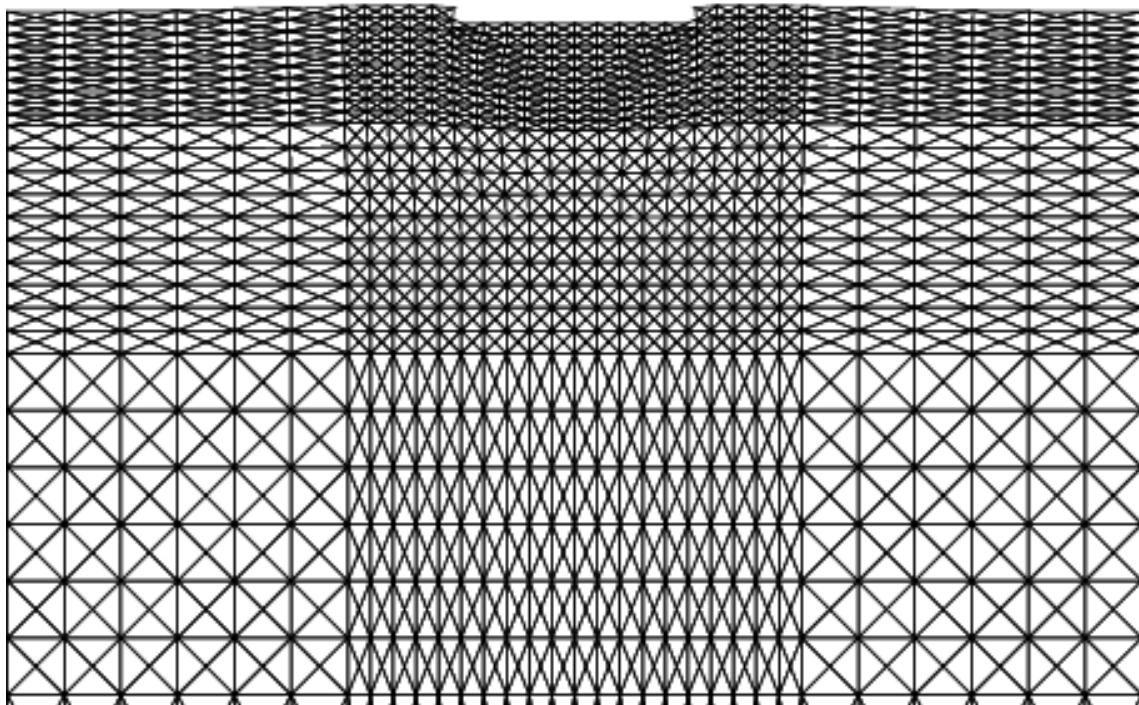


Figure B.9 Upper bound failure mechanism for COHESIVE_0.1_1.0 case (i.e. $c_{u1}/c_{u2} = 0.1$, $H/B = 1.0$).

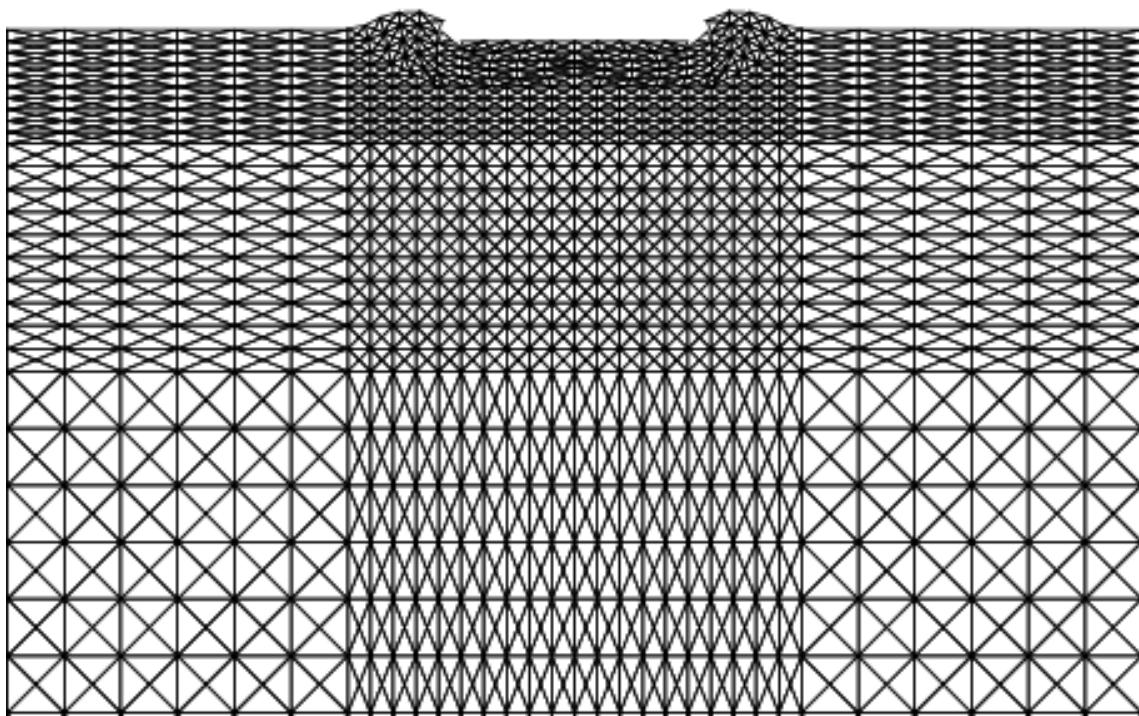


Figure B.10 Upper bound failure mechanism for COHESIVE_0.25_0.25 case (i.e. $c_{u1}/c_{u2} = 0.25$, $H/B = 0.25$).

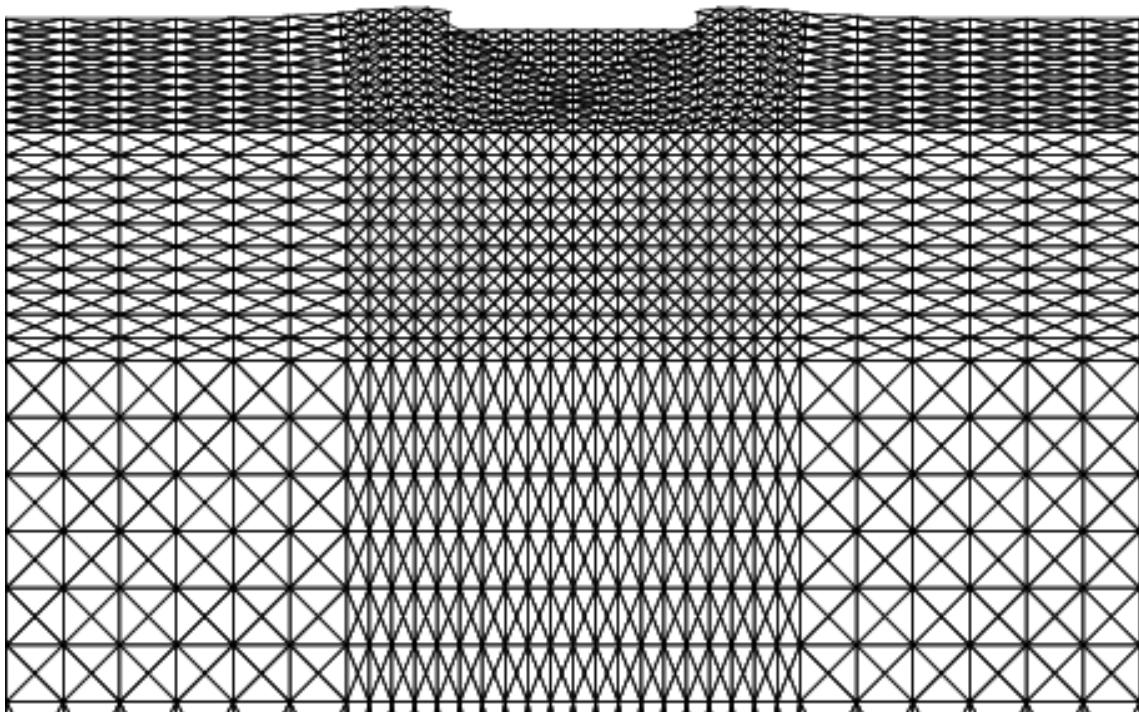


Figure B.11 Upper bound failure mechanism for COHESIVE_0.25_0.5 case (i.e. $c_{u1}/c_{u2} = 0.25$, $H/B = 0.5$).

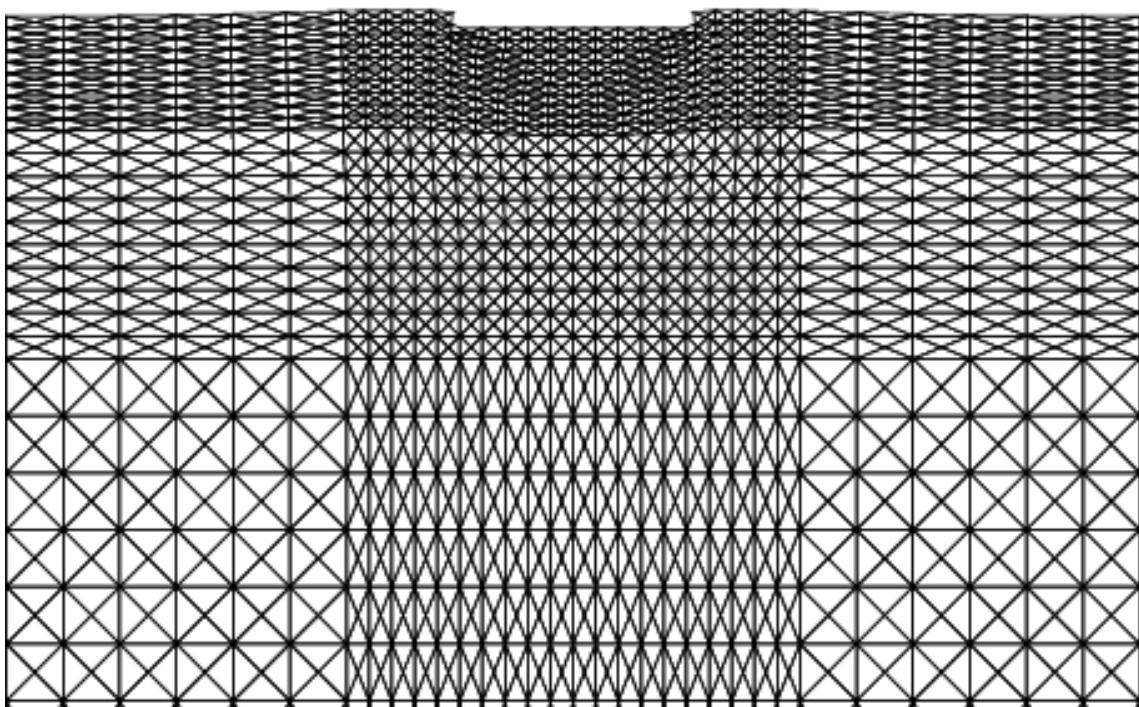


Figure B.12 Upper bound failure mechanism for COHESIVE_0.25_1.0 case (i.e. $c_{u1}/c_{u2} = 0.25$, $H/B = 1.0$).

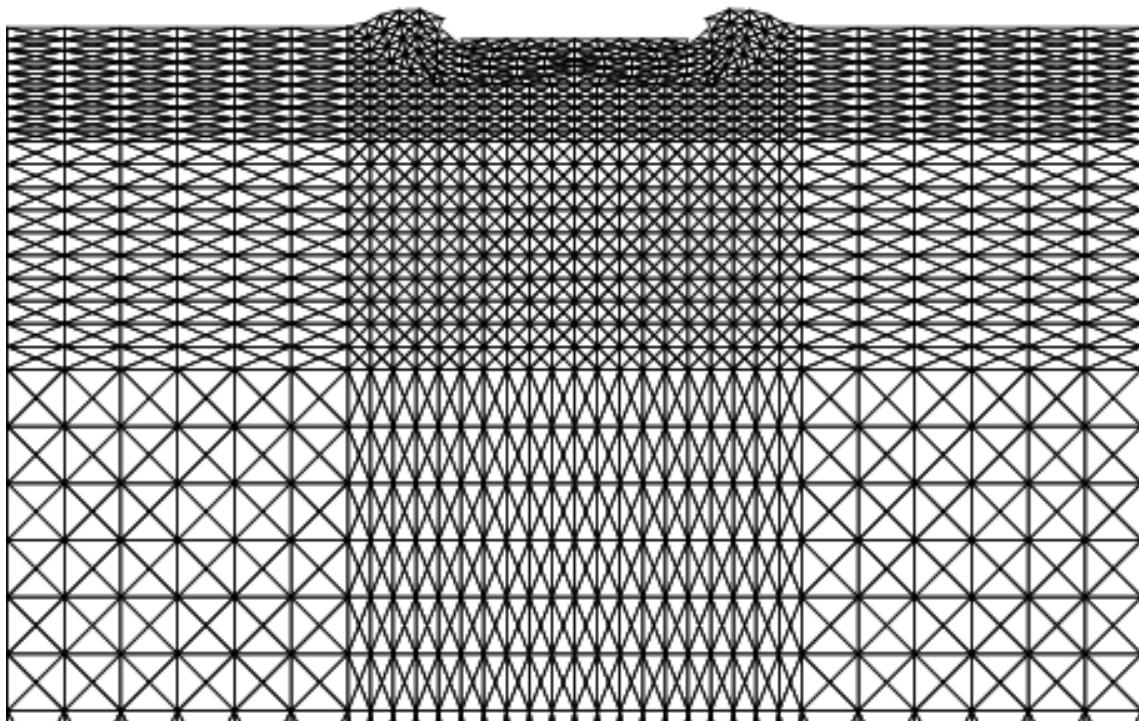


Figure B.13 Upper bound failure mechanism for COHESIVE_0.333_0.25 case (i.e. $c_{u1}/c_{u2} = 0.333$, $H/B = 0.25$).

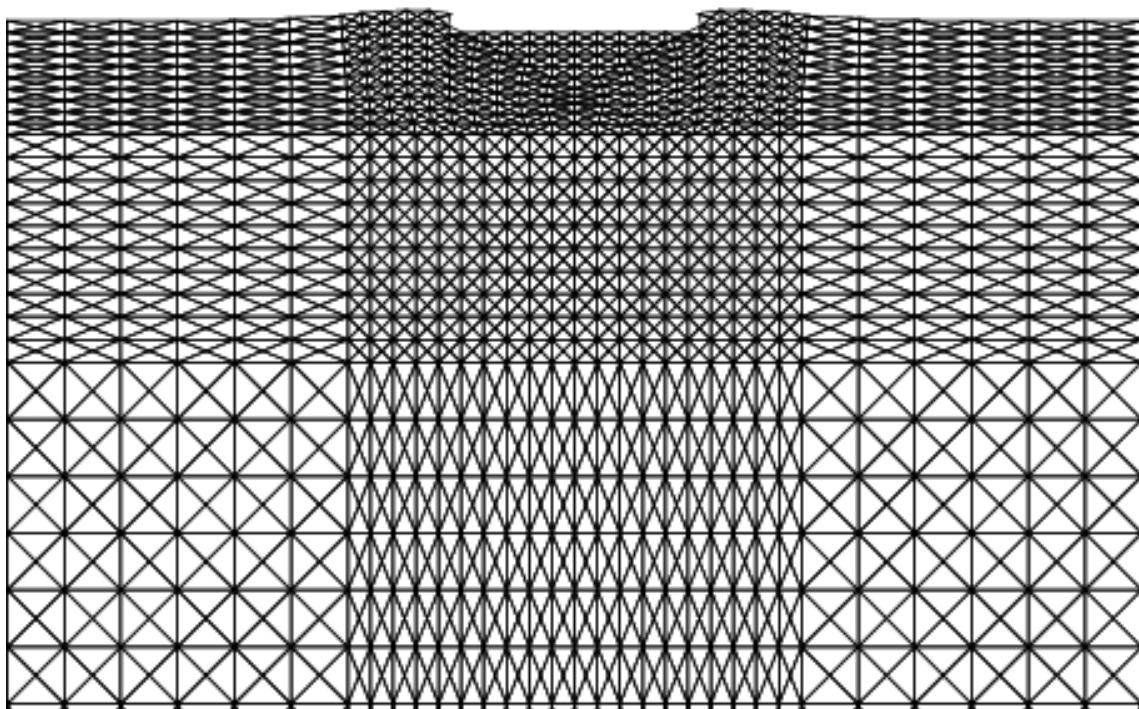


Figure B.14 Upper bound failure mechanism for COHESIVE_0.333_0.5 case (i.e. $c_{u1}/c_{u2} = 0.333$, $H/B = 0.5$).

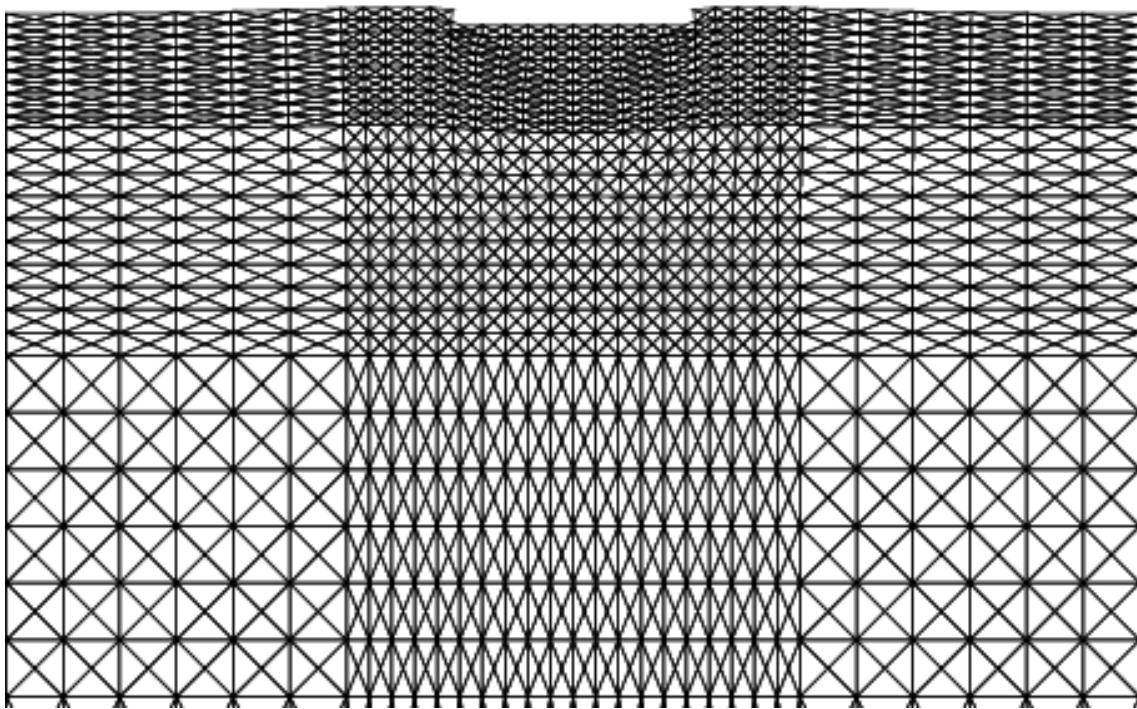


Figure B.15 Upper bound failure mechanism for COHESIVE_0.333_1.0 case (i.e. $c_{u1}/c_{u2} = 0.33$, $H/B = 1.0$).

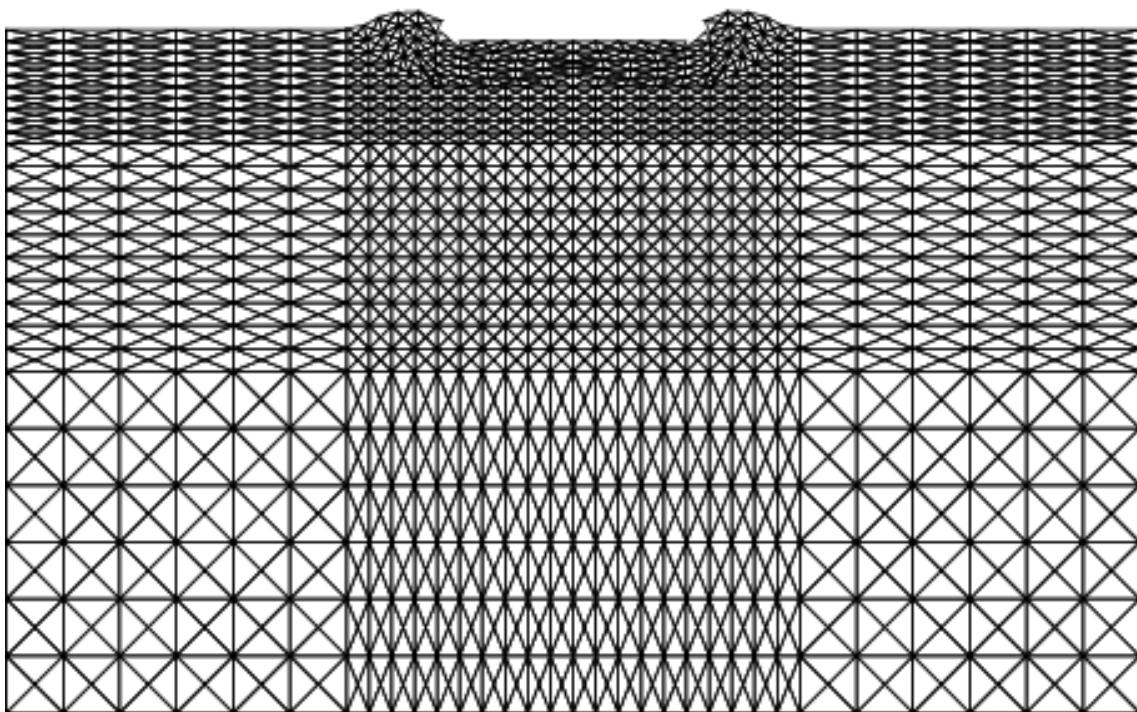


Figure B.16 Upper bound failure mechanism for COHESIVE_0.5_0.25 case (i.e. $c_{u1}/c_{u2} = 0.5$, $H/B = 0.25$).

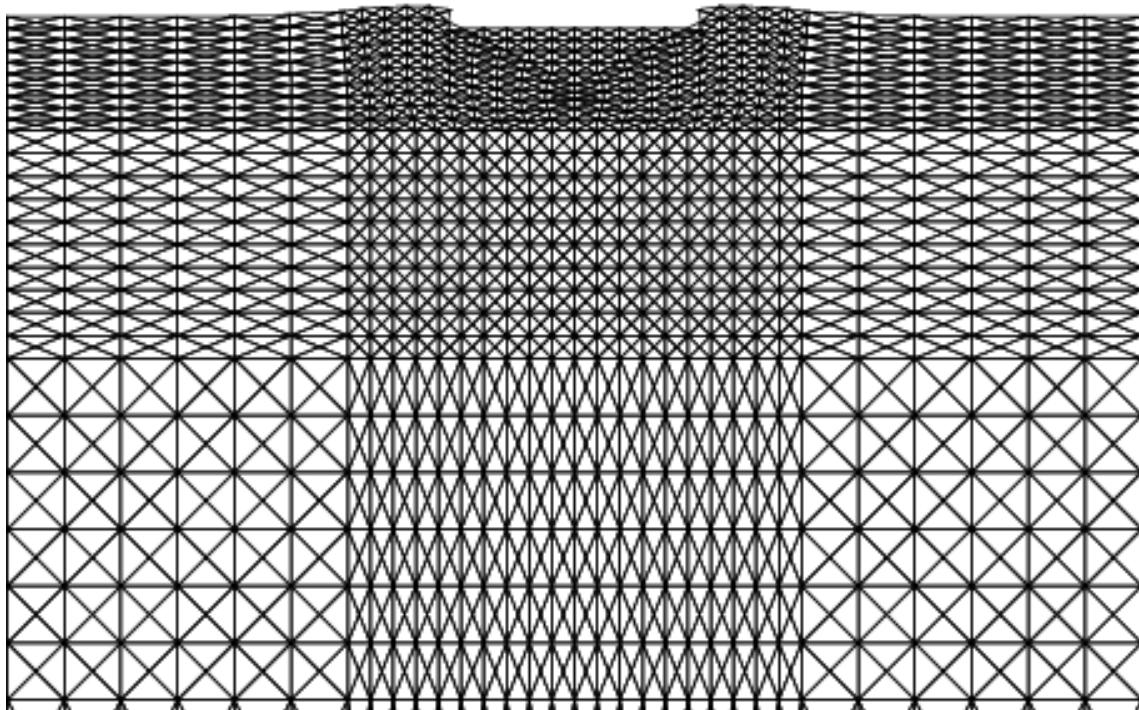


Figure B.17 Upper bound failure mechanism for COHESIVE_0.5_0.5 case (i.e. $c_{u1}/c_{u2} = 0.5, H/B = 0.5$).

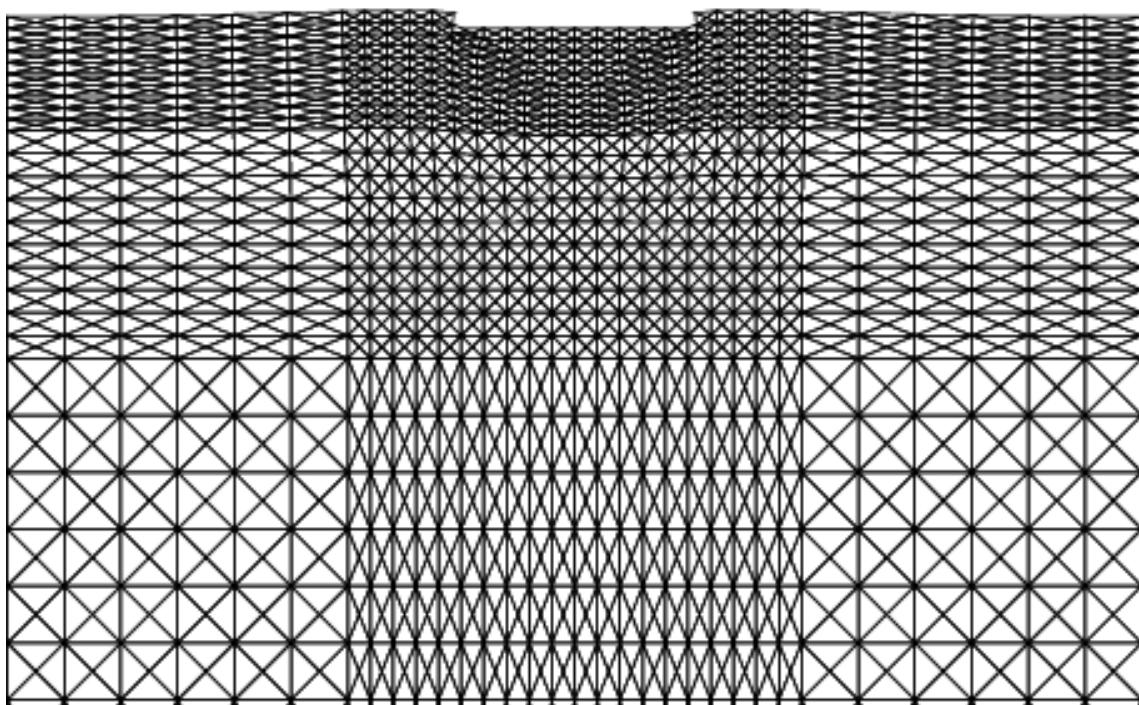


Figure B.18 Upper bound failure mechanism for COHESIVE_0.5_1.0 case (i.e. $c_{u1}/c_{u2} = 0.5, H/B = 1.0$).

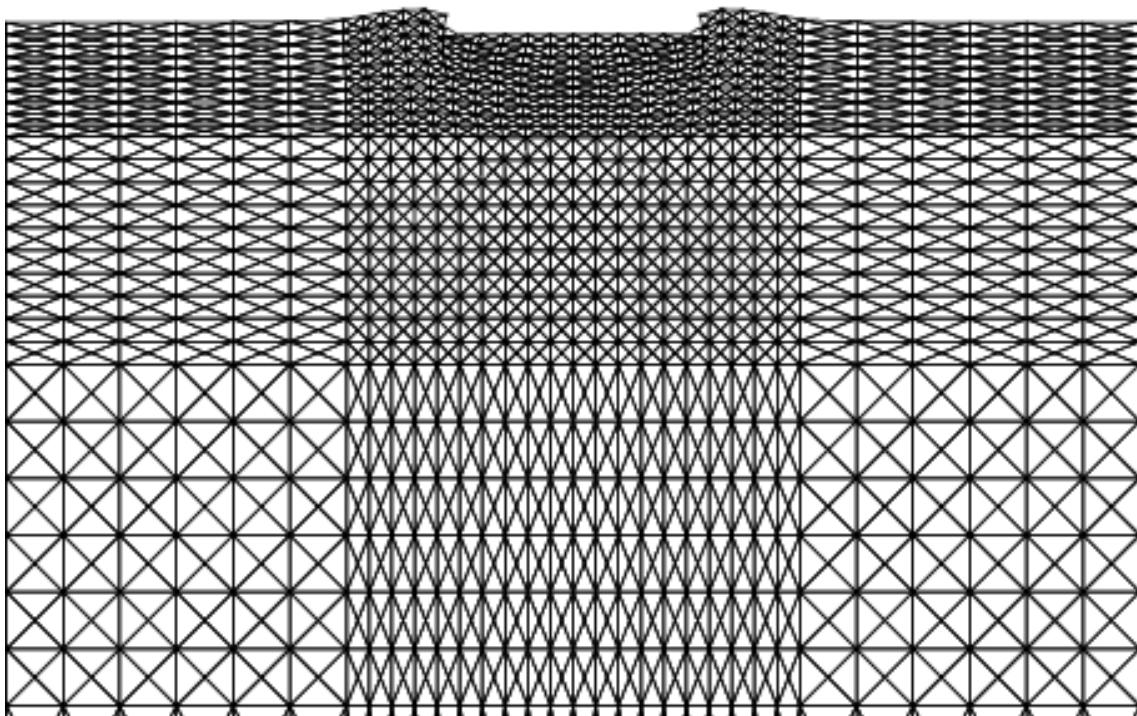


Figure B.19 Upper bound failure mechanism for COHESIVE_0.75_0.25 case (i.e. $c_{u1}/c_{u2} = 0.75$, $H/B = 0.25$).

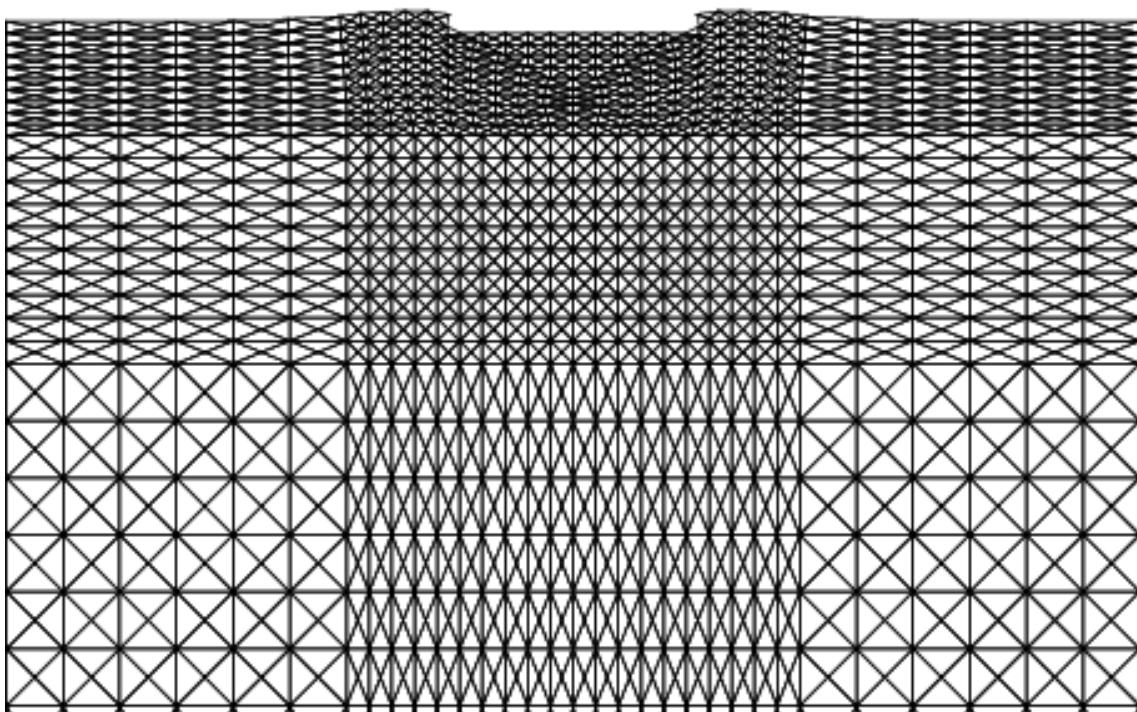


Figure B.20 Upper bound failure mechanism for COHESIVE_0.75_0.5 case (i.e. $c_{u1}/c_{u2} = 0.75$, $H/B = 0.5$).

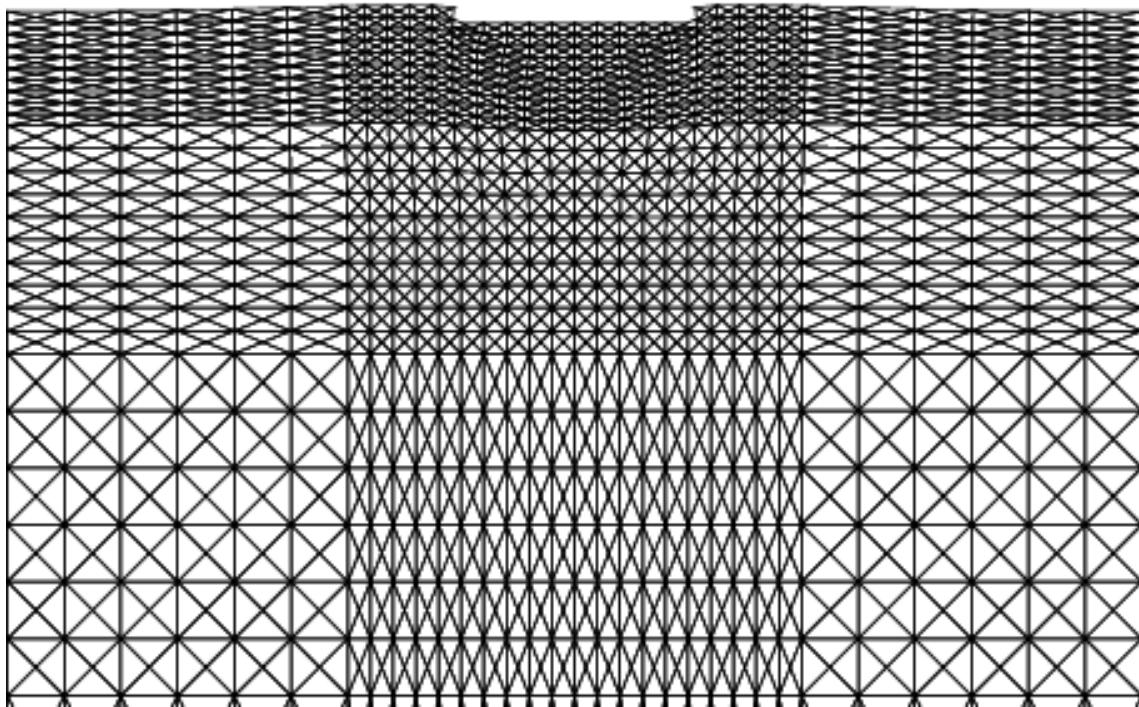


Figure B.21 Upper bound failure mechanism for COHESIVE_0.75_1.0 case (i.e. $c_{u1}/c_{u2} = 0.75, H/B = 1.0$).

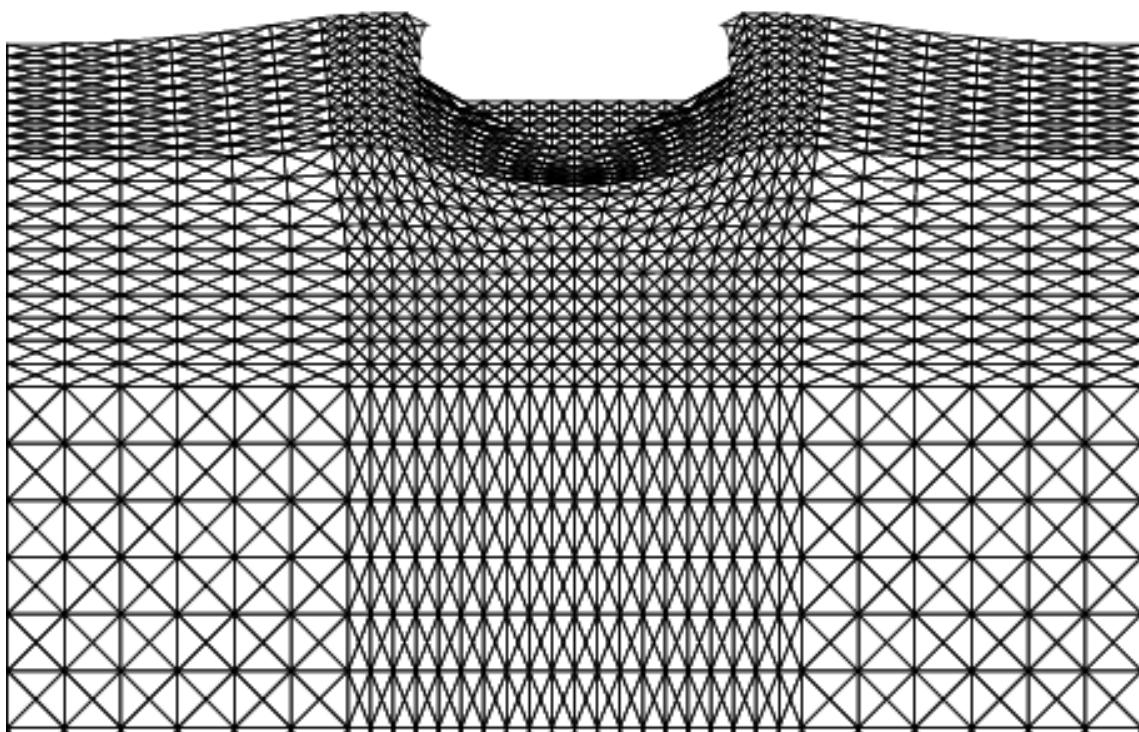


Figure B.22 Upper bound failure mechanism for single-layered deterministic homogeneous case (i.e. $c_{u1}/c_{u2} = 1.0$)

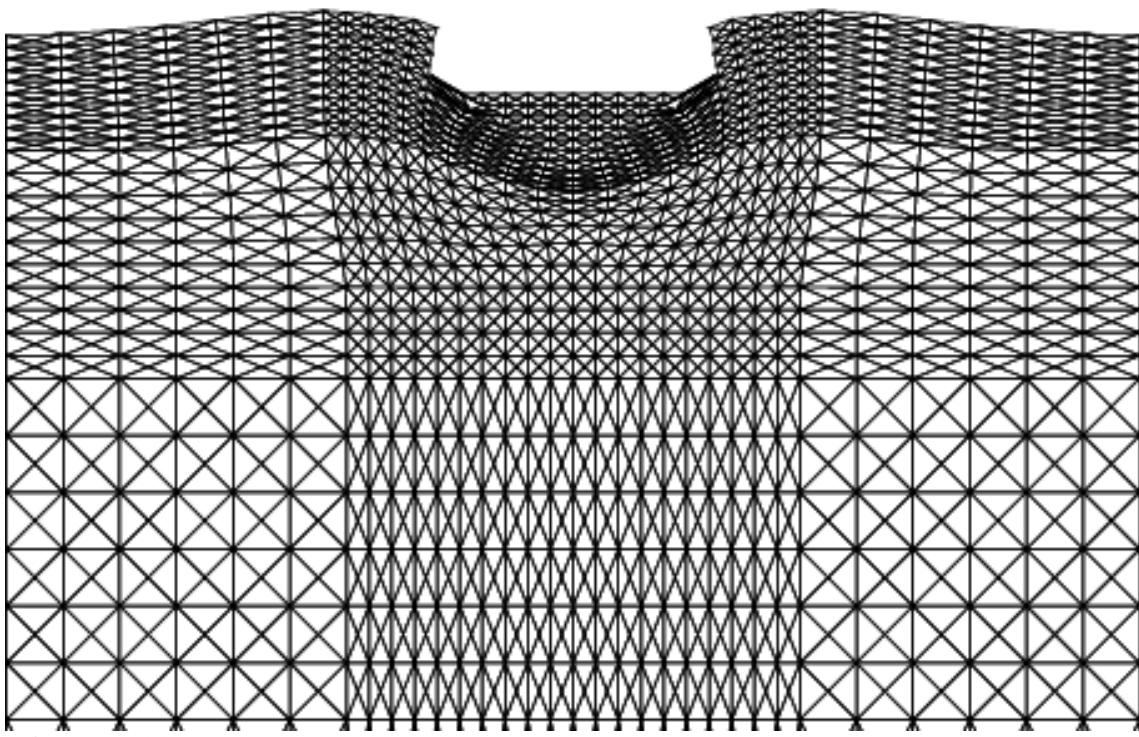


Figure B.23 Upper bound failure mechanism for COHESIVE_1.333_0.25 case (i.e. $c_{u1}/c_{u2} = 1.33$, $H/B = 0.25$).

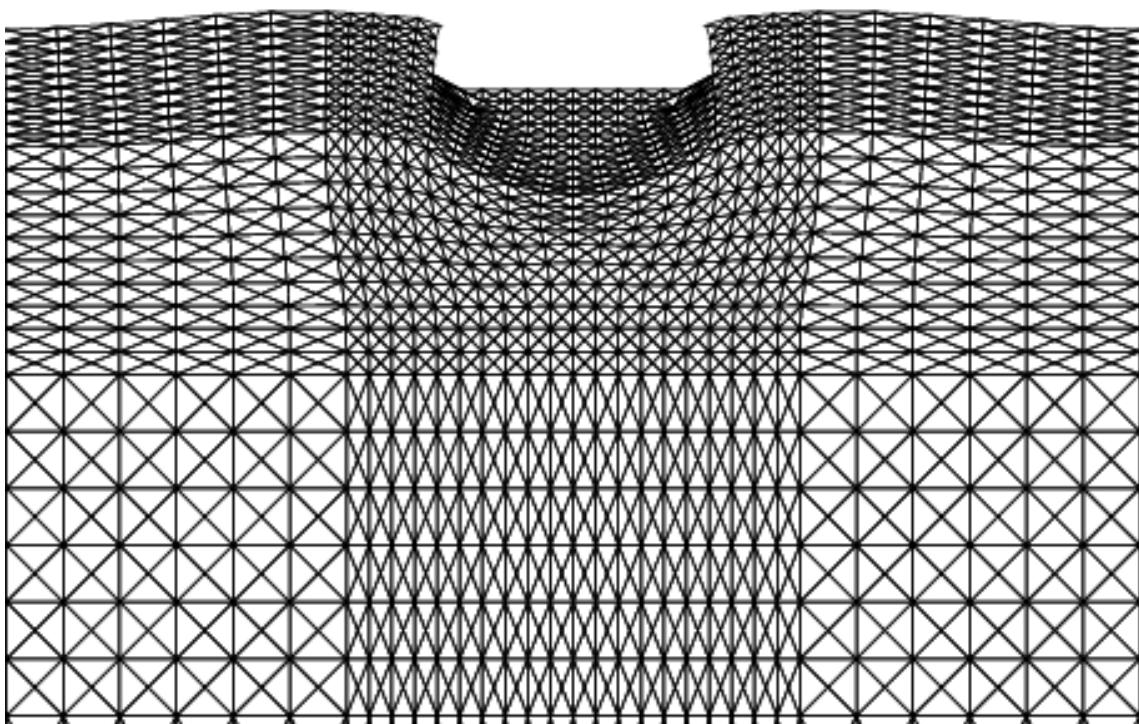


Figure B.24 Upper bound failure mechanism for COHESIVE_1.333_0.5 case (i.e. $c_{u1}/c_{u2} = 1.33$, $H/B = 0.5$).

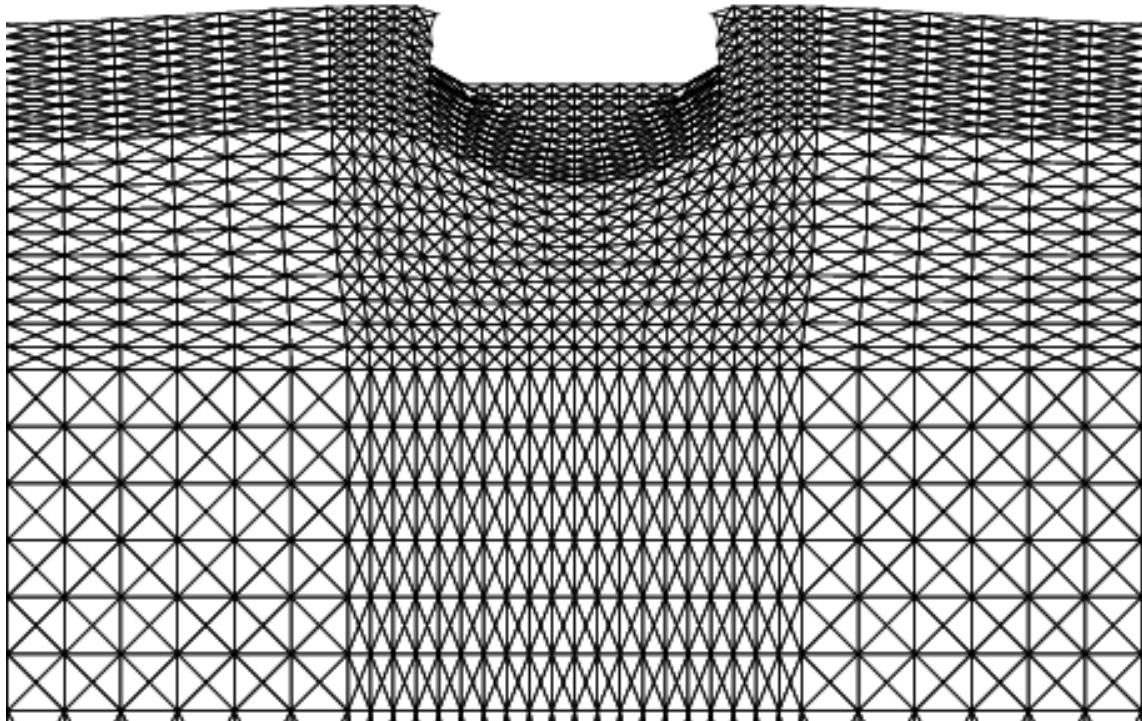


Figure B.25 Upper bound failure mechanism for COHESIVE_1.333_1.0 case (i.e. $c_{u1}/c_{u2} = 1.33$, $H/B = 1.0$).

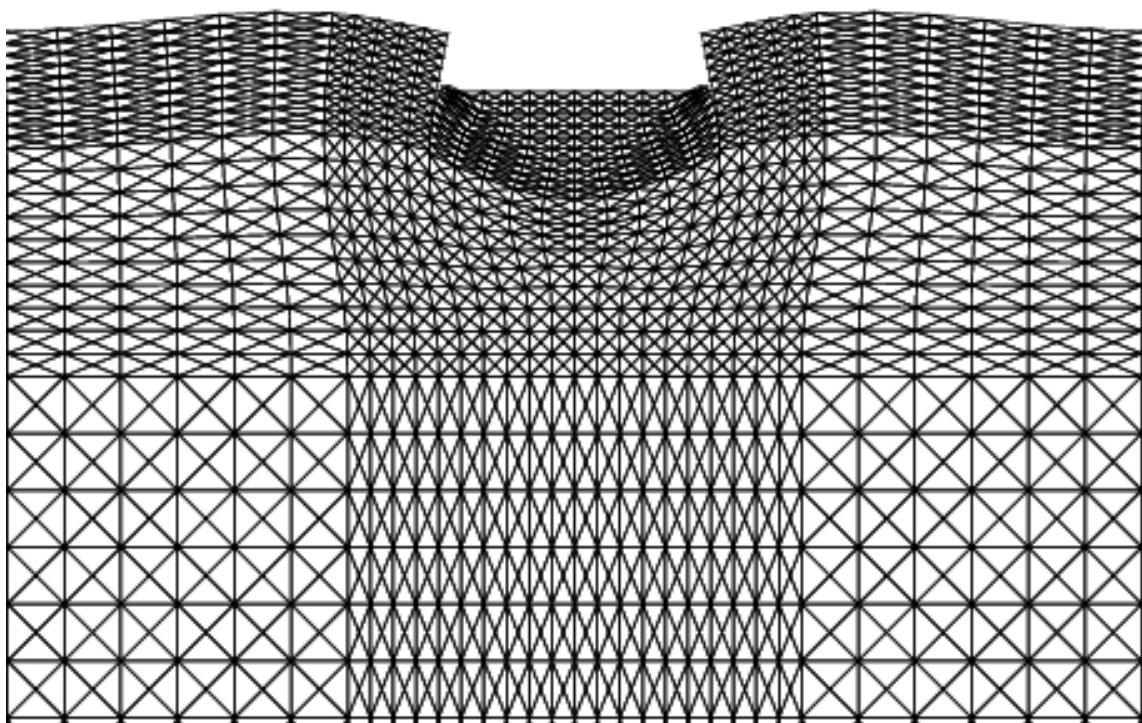


Figure B.26 Upper bound failure mechanism for COHESIVE_2.0_0.25 case (i.e. $c_{u1}/c_{u2} = 2.0$, $H/B = 0.25$).

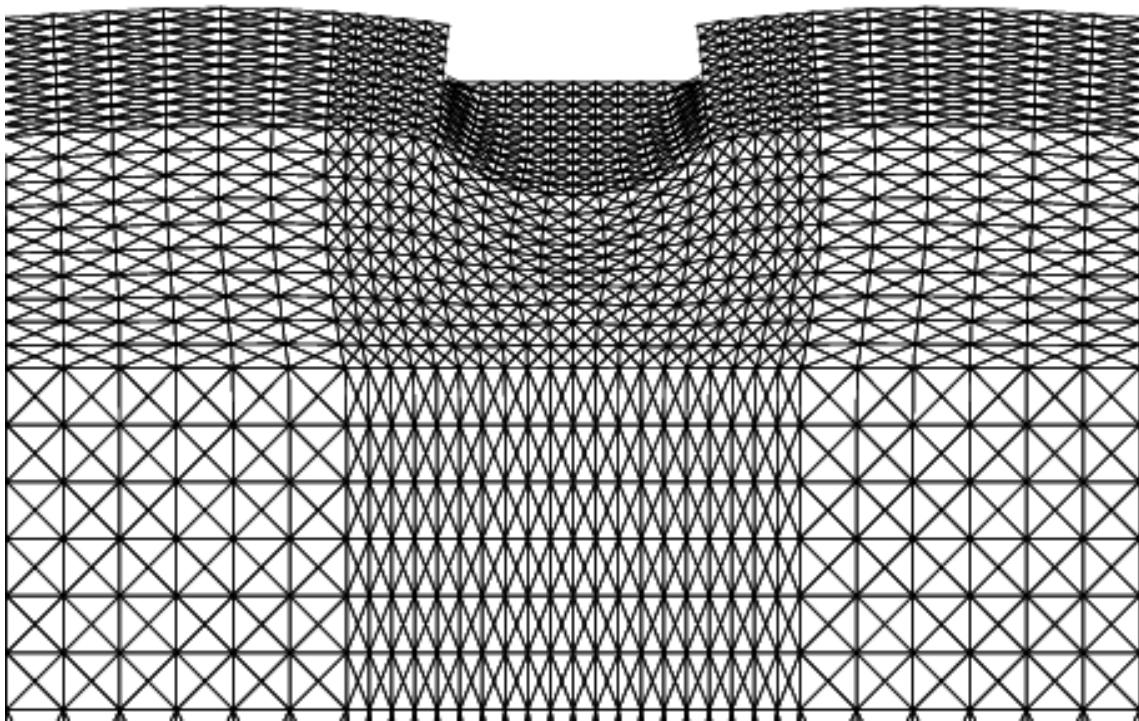


Figure B.27 Upper bound failure mechanism for COHESIVE_2.0_0.5 case (i.e. $c_{u1}/c_{u2} = 2.0$, $H/B = 0.5$).

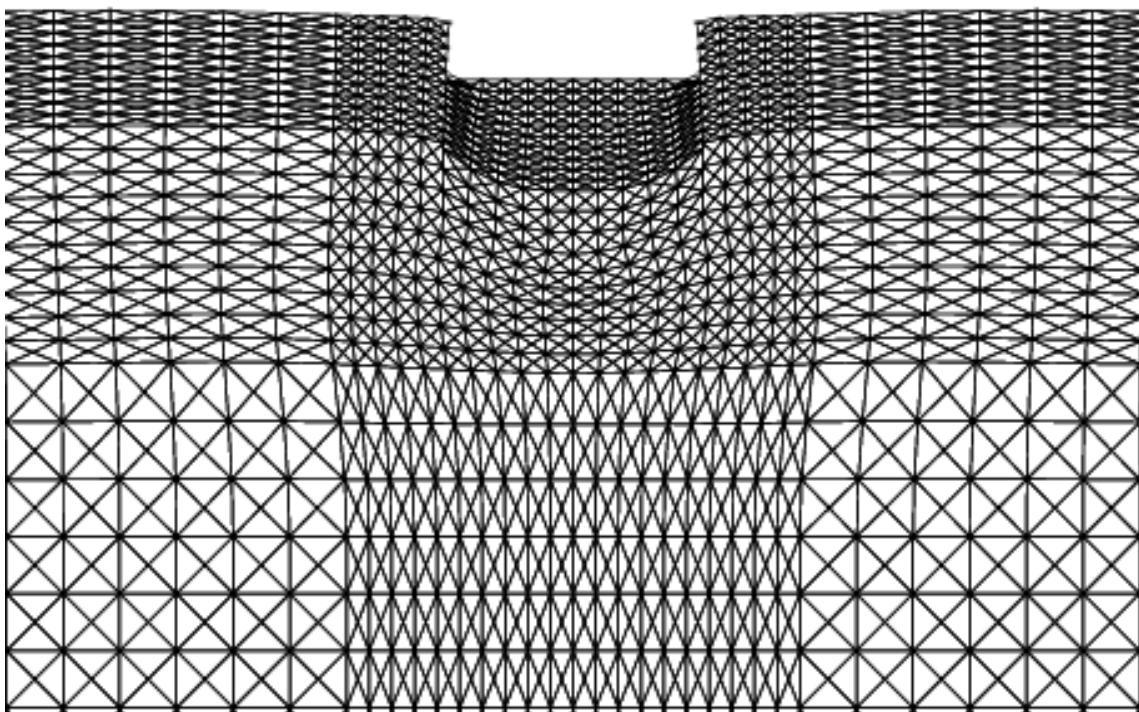


Figure B.28 Upper bound failure mechanism for COHESIVE_2.0_1.0 case (i.e. $c_{u1}/c_{u2} = 2.0$, $H/B = 1.0$).

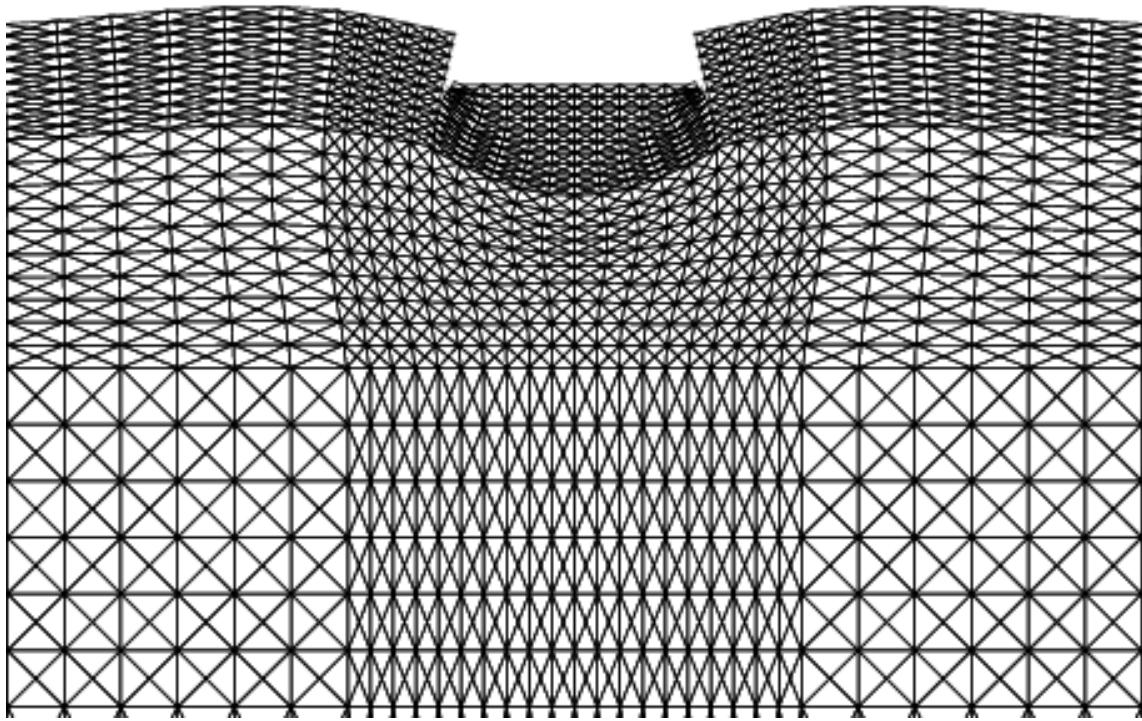


Figure B.29 Upper bound failure mechanism for COHESIVE_3.0_0.25 case (i.e. $c_{u1}/c_{u2} = 3.0$, $H/B = 0.25$).

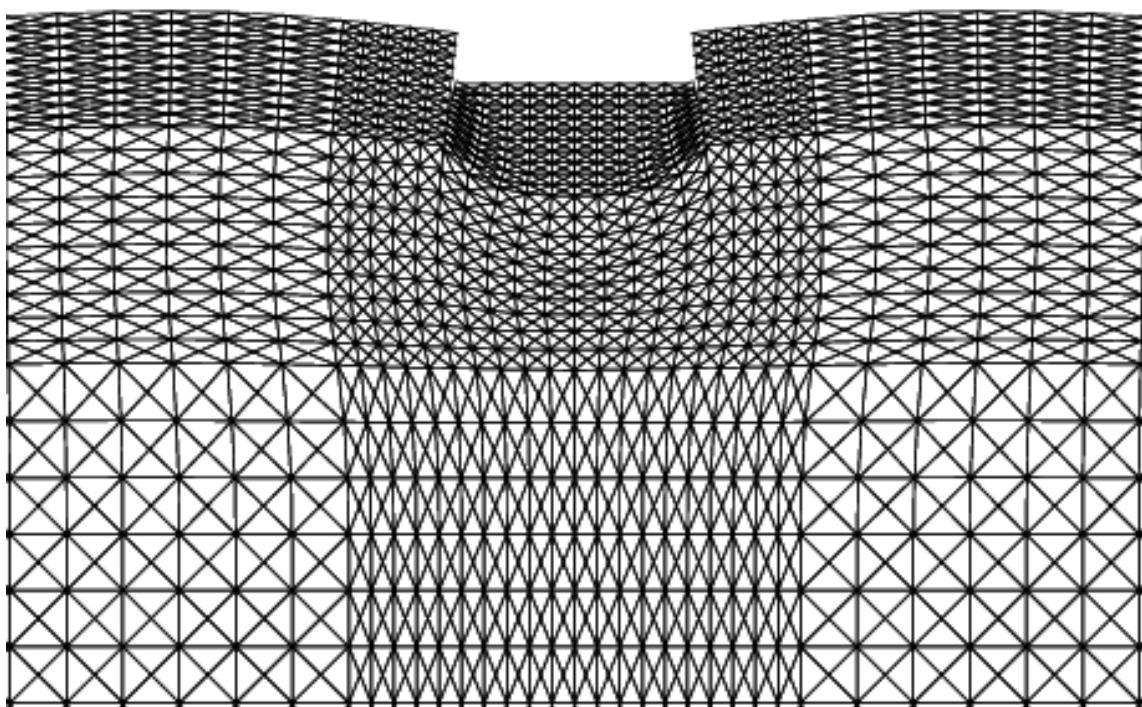


Figure B.30 Upper bound failure mechanism for COHESIVE_3.0_0.5 case (i.e. $c_{u1}/c_{u2} = 3.0$, $H/B = 0.5$).

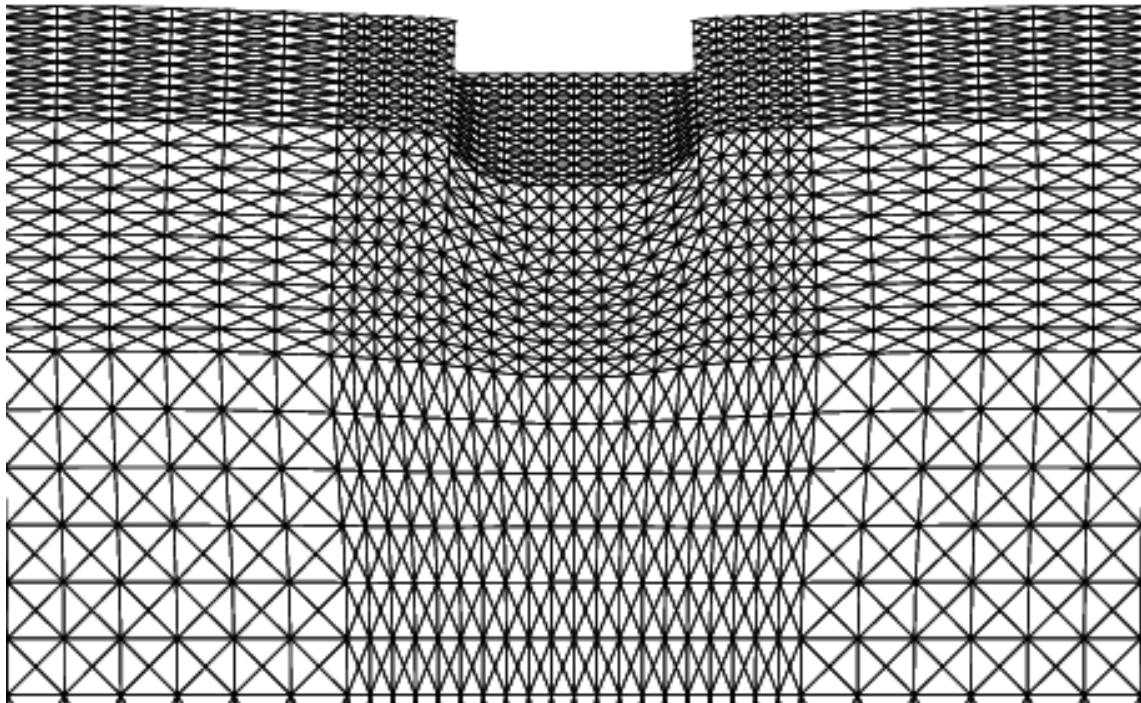


Figure B.31 Upper bound failure mechanism for COHESIVE_3.0_1.0 case (i.e.
 $c_{u1}/c_{u2} = 3.0, H/B = 1.0$).

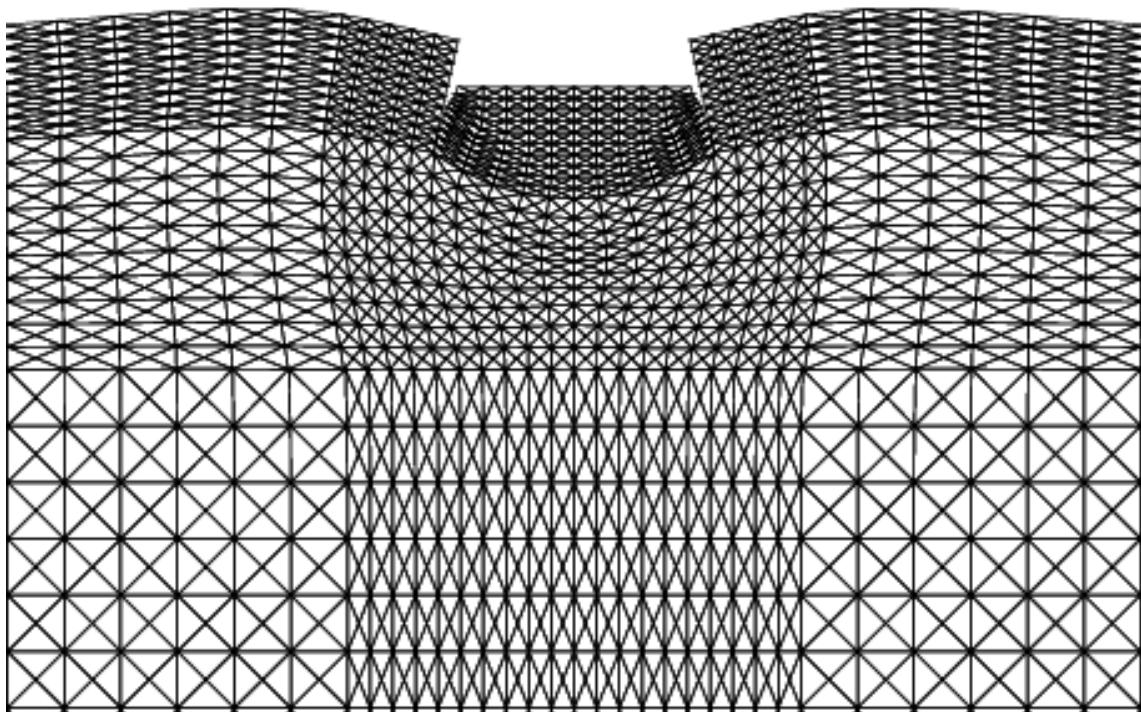


Figure B.32 Upper bound failure mechanism for COHESIVE_4.0_0.25 case (i.e.
 $c_{u1}/c_{u2} = 4.0, H/B = 0.25$).

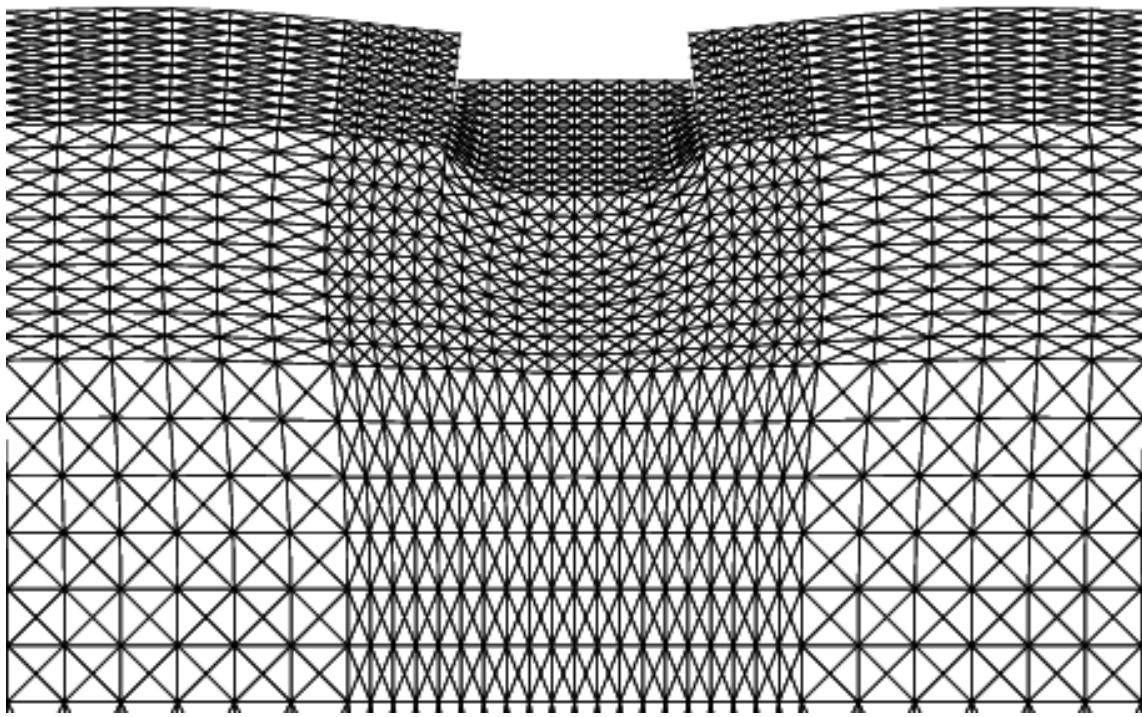


Figure B.33 Upper bound failure mechanism for COHESIVE_4.0_0.5 case (i.e. $c_{u1}/c_{u2} = 4.0$, $H/B = 0.5$).

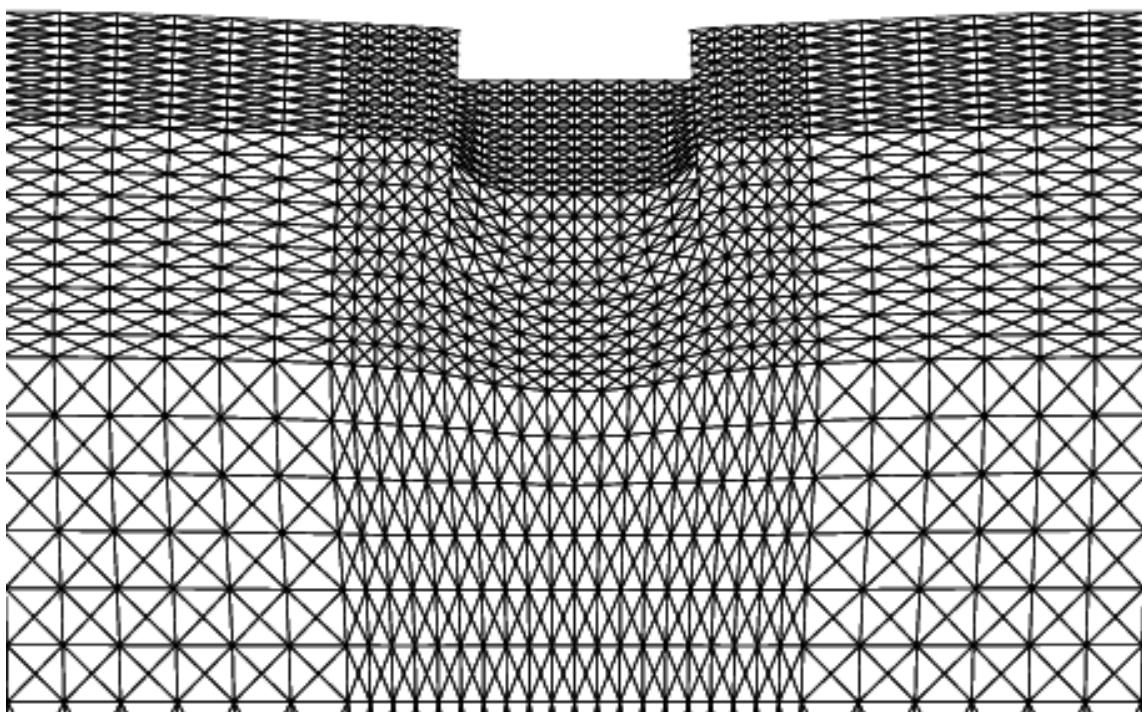


Figure B.34 Upper bound failure mechanism for COHESIVE_4.0_1.0 case (i.e. $c_{u1}/c_{u2} = 4.0$, $H/B = 1.0$).

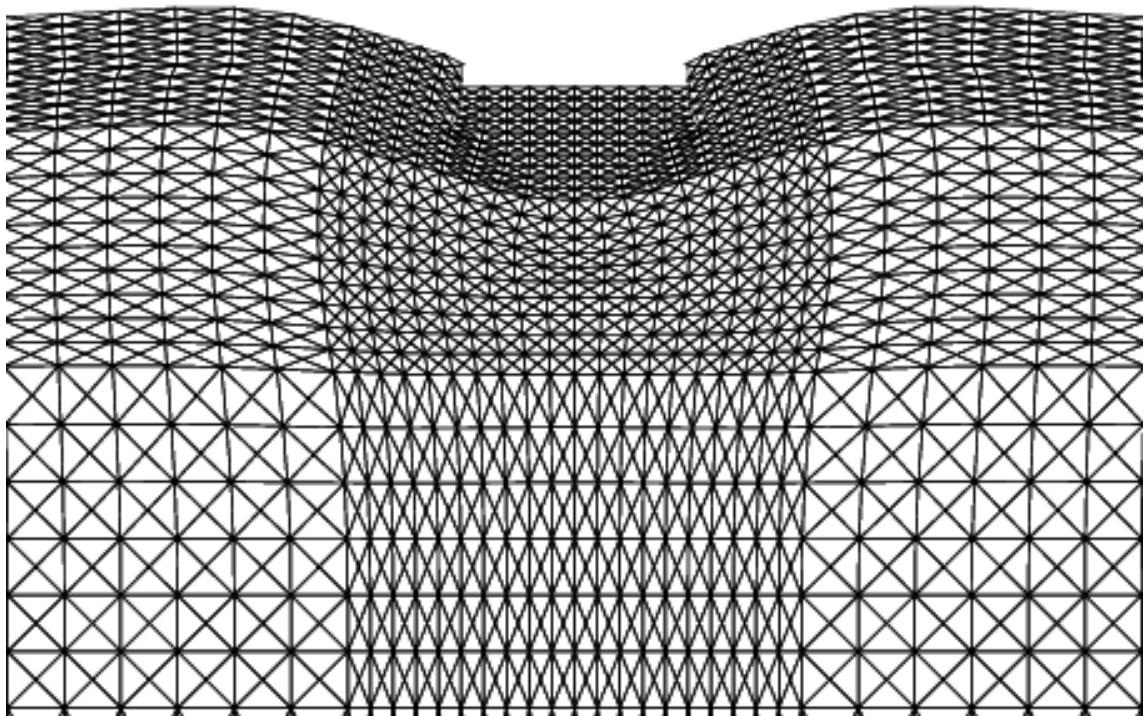


Figure B.35 Upper bound failure mechanism for COHESIVE_10.0_0.25 case (i.e. $c_{u1}/c_{u2} = 10.0$, $H/B = 0.25$).

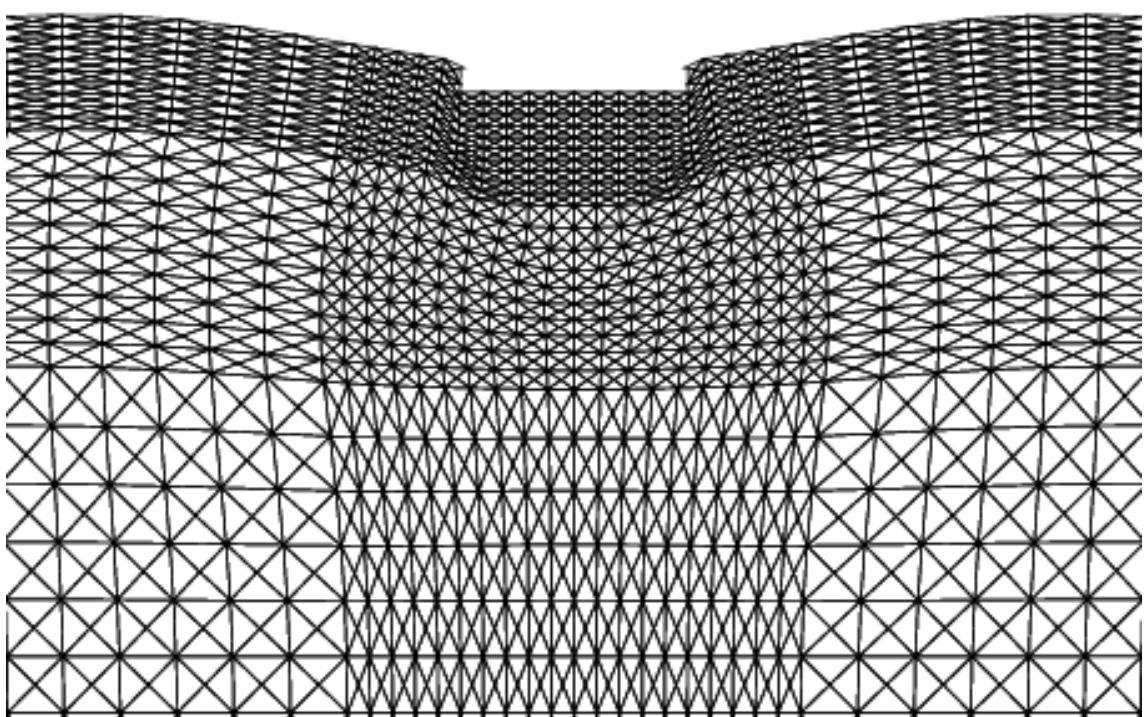


Figure B.36 Upper bound failure mechanism for COHESIVE_10.0_0.5 case (i.e. $c_{u1}/c_{u2} = 10.0$, $H/B = 0.5$).

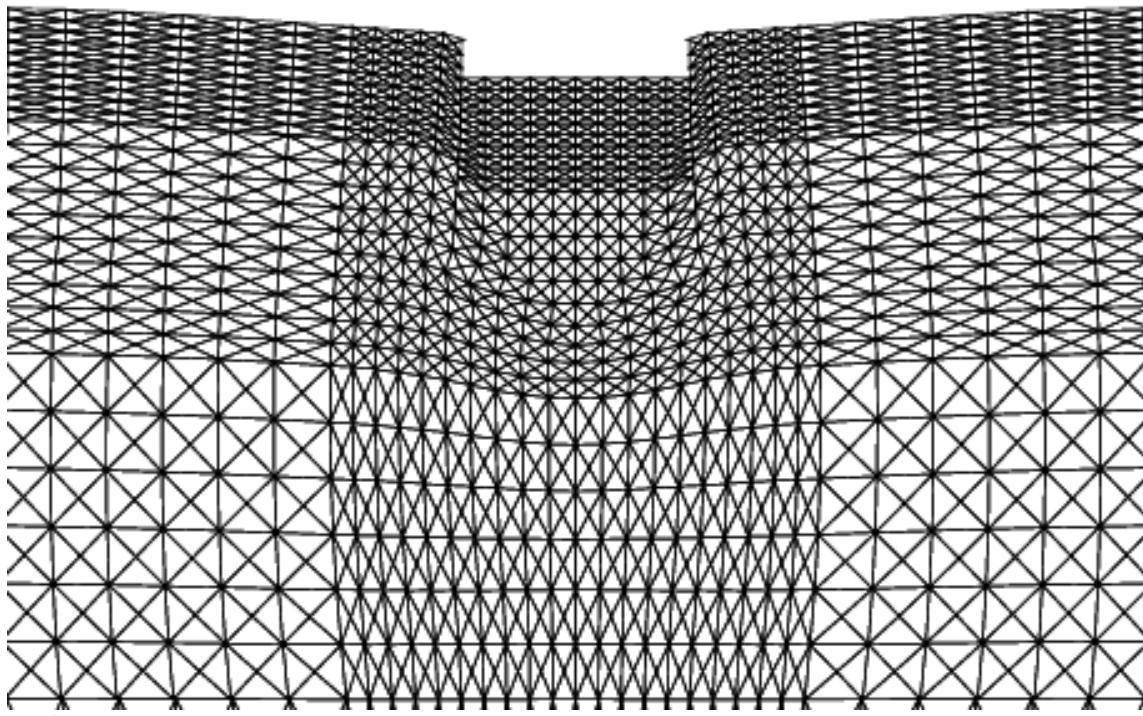


Figure B.37 Upper bound failure mechanism for COHESIVE_10.0_1.0 case (i.e. $c_{u1}/c_{u2} = 10.0$, $H/B = 1.0$).

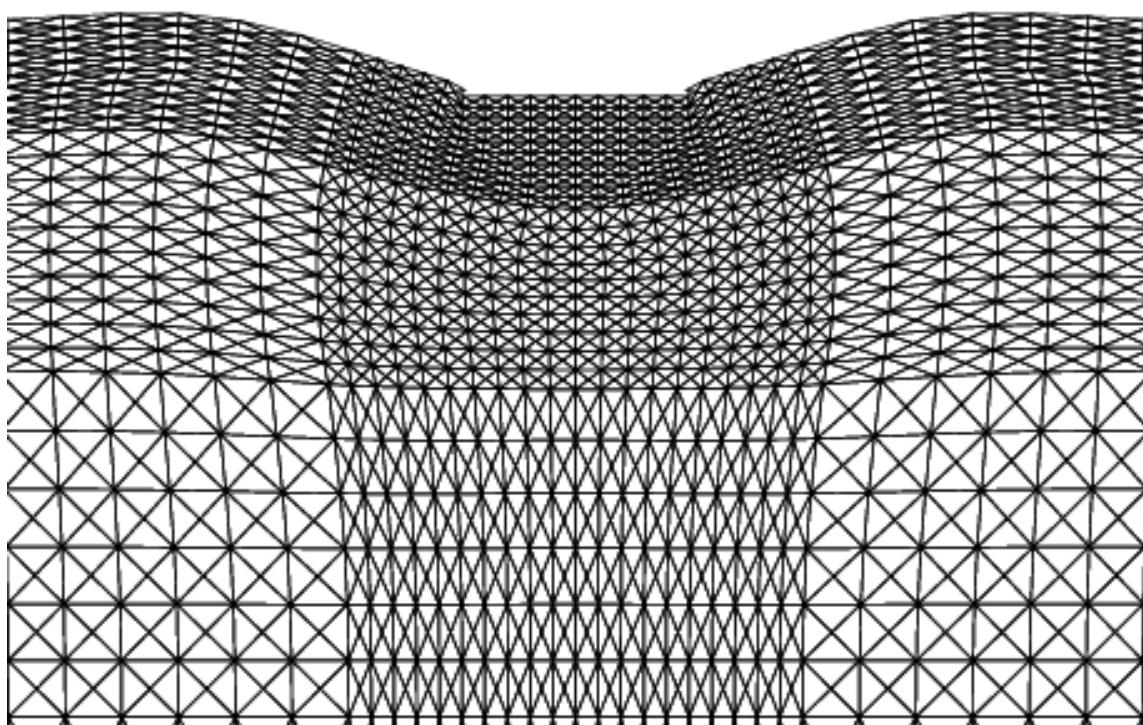


Figure B.38 Upper bound failure mechanism for COHESIVE_20.0_0.25 case (i.e. $c_{u1}/c_{u2} = 20.0$, $H/B = 0.25$).

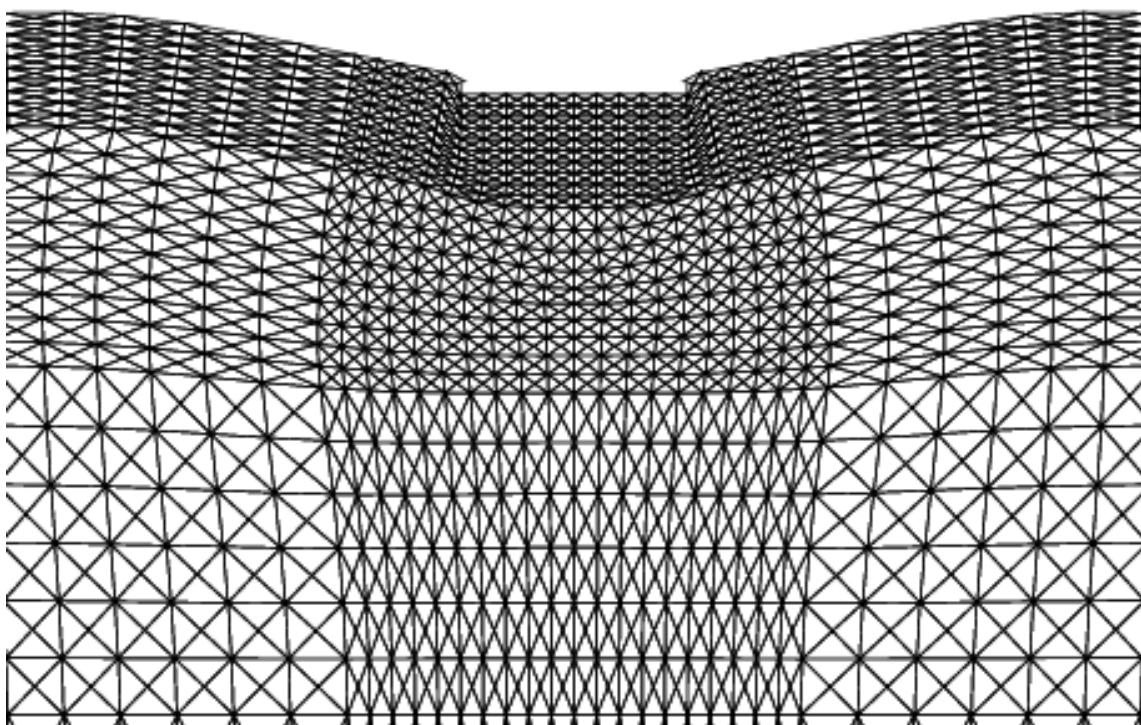


Figure B.39 Upper bound failure mechanism for COHESIVE_20.0_0.5 case (i.e. $c_{u1}/c_{u2} = 20.0$, $H/B = 0.5$).

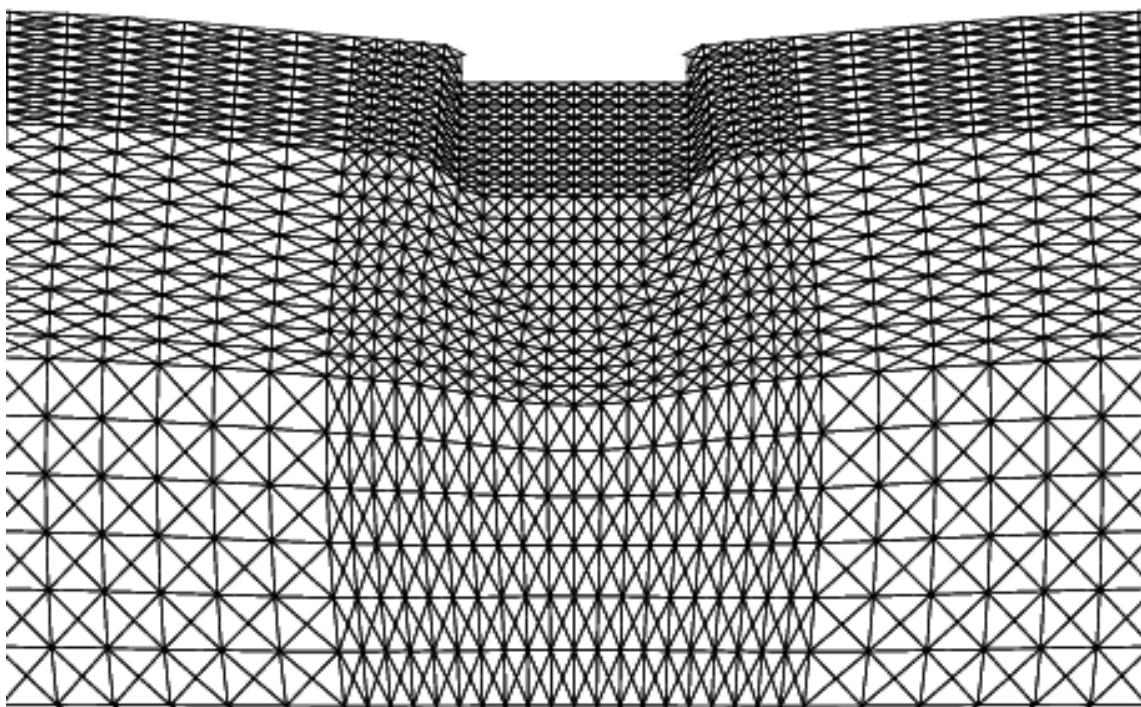


Figure B.40 Upper bound failure mechanism for COHESIVE_20.0_1.0 case (i.e. $c_{u1}/c_{u2} = 20.0$, $H/B = 1.0$).

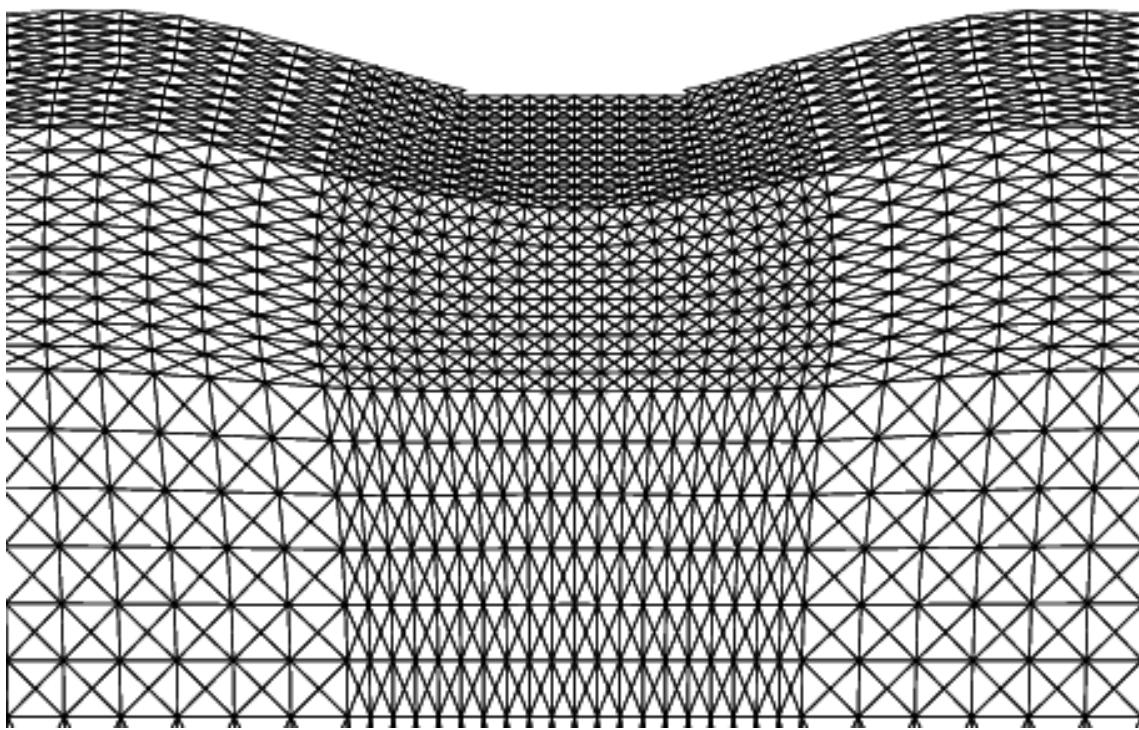


Figure B.41 Upper bound failure mechanism for COHESIVE_40.0_0.25 case (i.e. $c_{u1}/c_{u2} = 40.0$, $H/B = 0.25$).

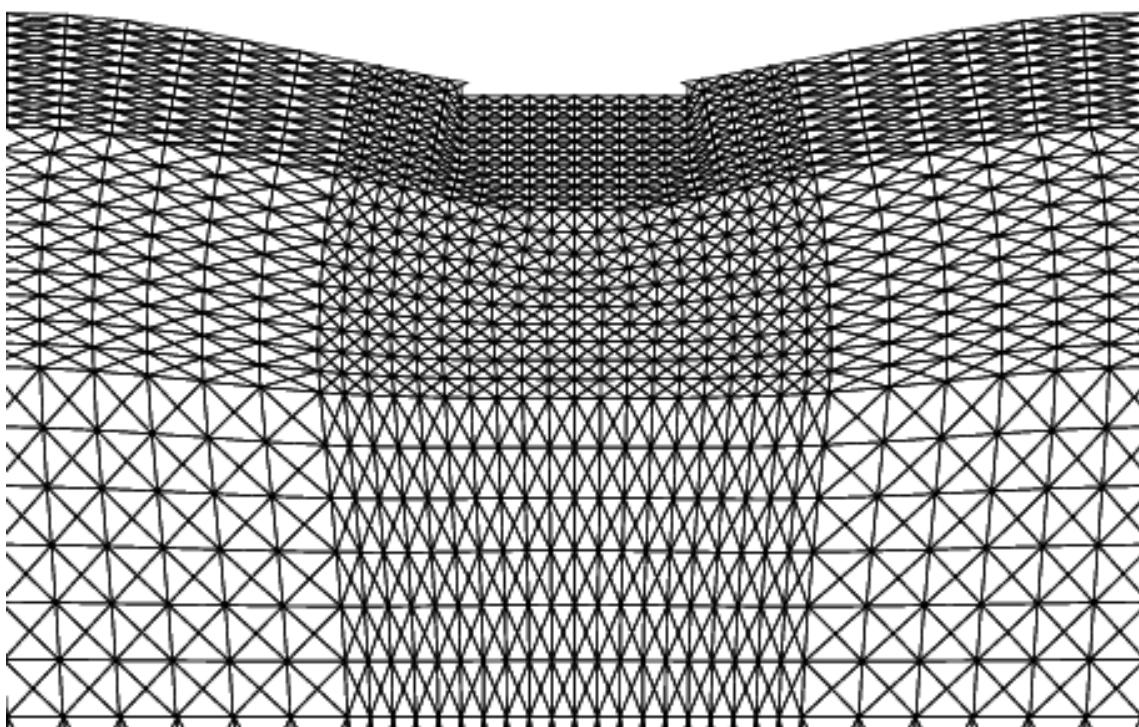


Figure B.42 Upper bound failure mechanism for COHESIVE_40.0_0.5 case (i.e. $c_{u1}/c_{u2} = 40.0$, $H/B = 0.5$).

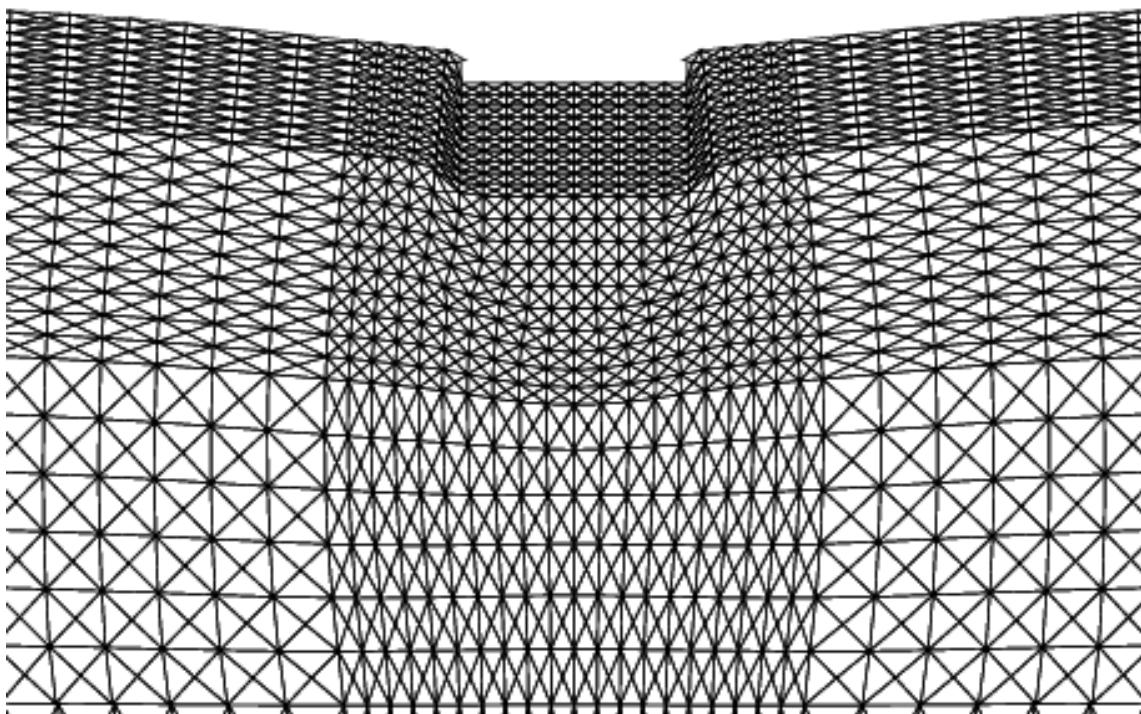


Figure B.43 Upper bound failure mechanism for COHESIVE_40.0_1.0 case (i.e. $c_{u1}/c_{u2} = 40.0$, $H/B = 1.0$).

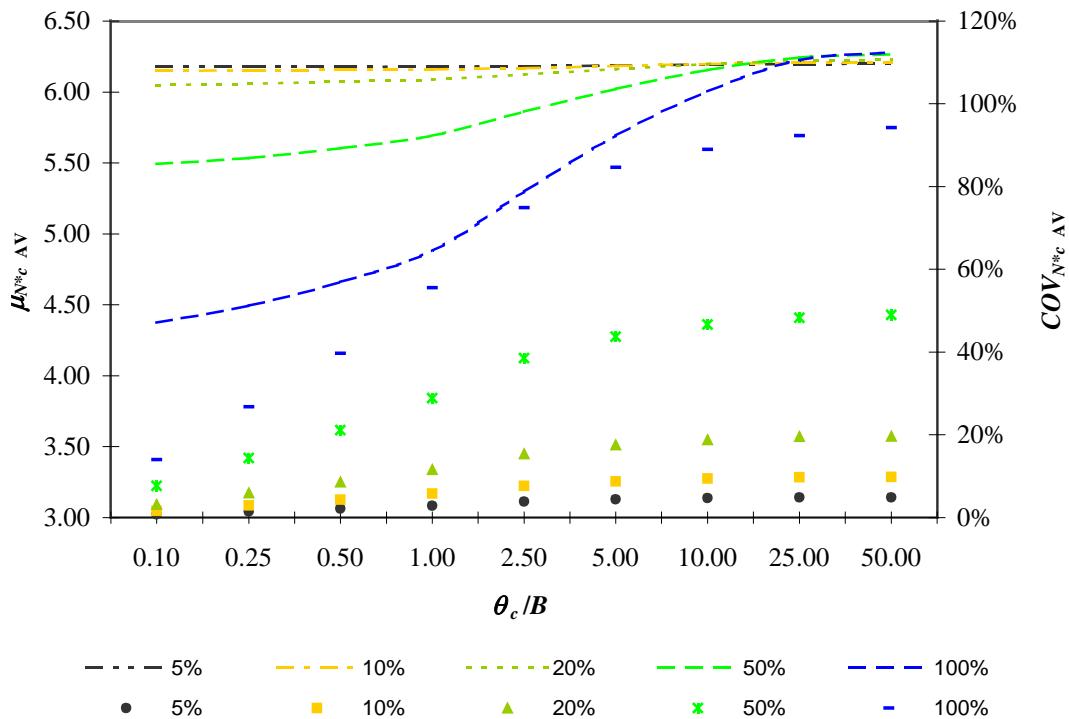


Figure B.44 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_0.025_0.25 case (where $\mu_{c1}/\mu_{c2} = 0.025$ and $H/B = 0.25$).

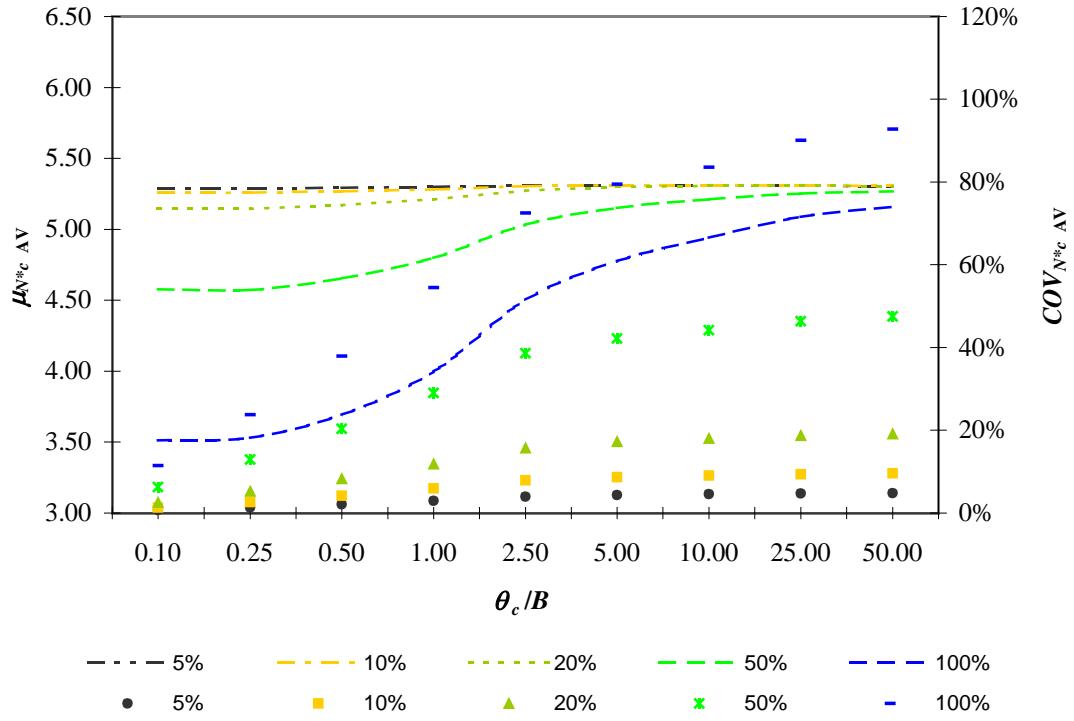


Figure B.45 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_0.025_0.5 case (where $\mu_{c1}/\mu_{c2} = 0.025$ and $H/B = 0.5$).

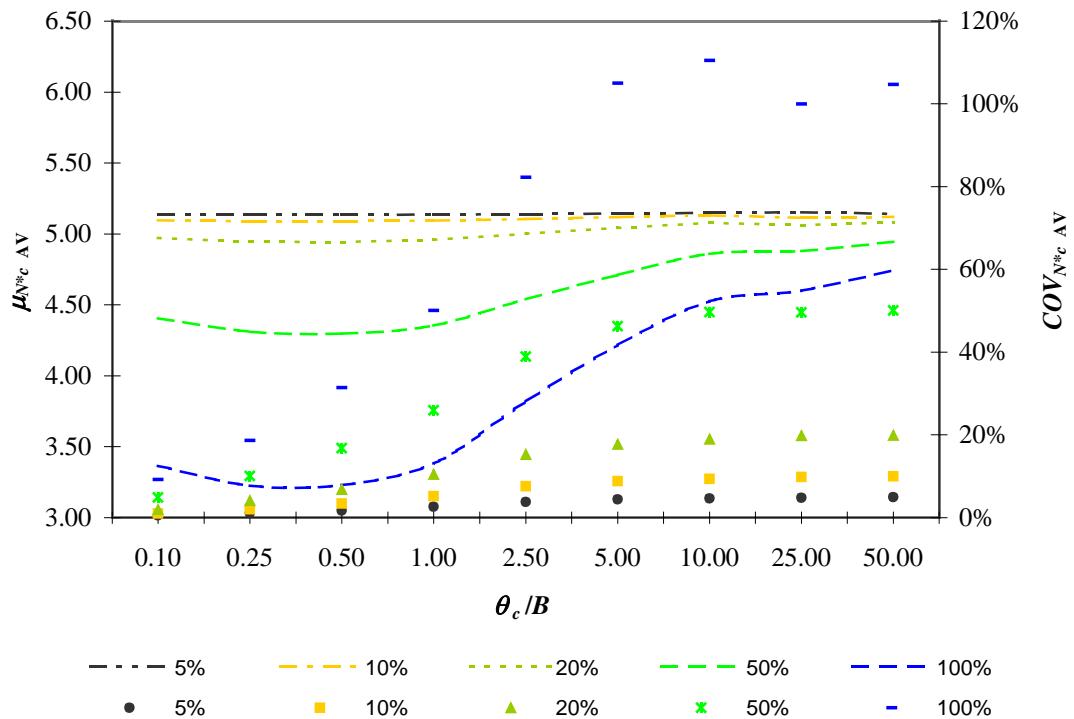


Figure B.46 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_0.025_1.0 case (where $\mu_{c1}/\mu_{c2} = 0.025$ and $H/B = 1.0$).

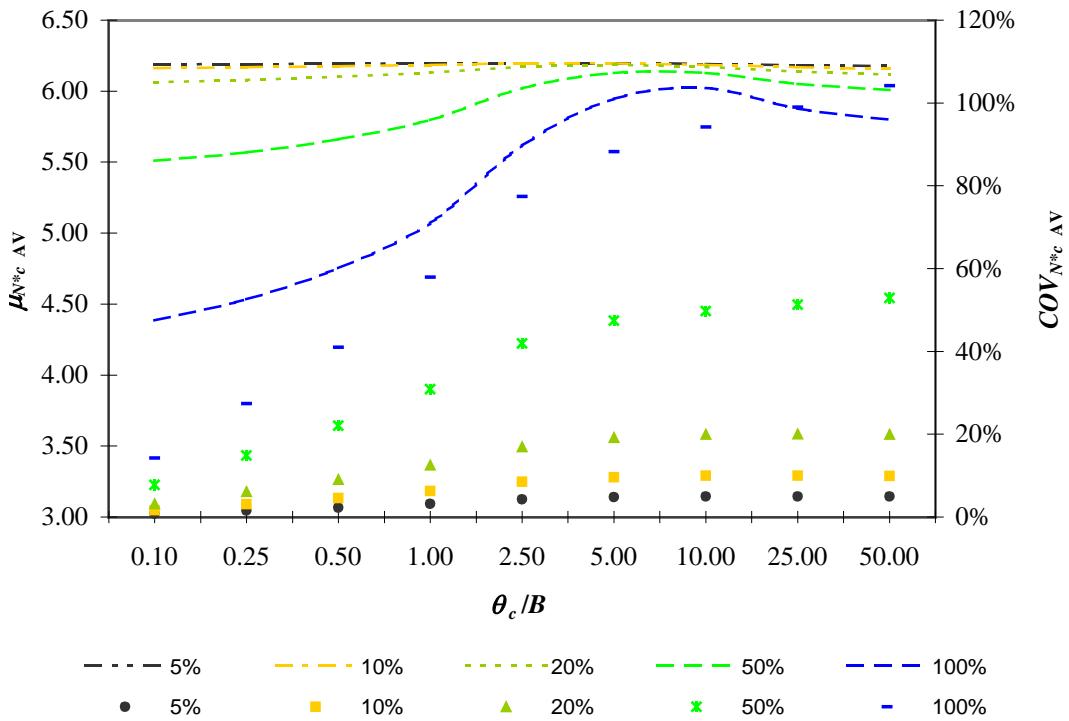


Figure B.47 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_0.05_0.25 case (where $\mu_{c1}/\mu_{c2} = 0.05$ and $H/B = 0.25$).

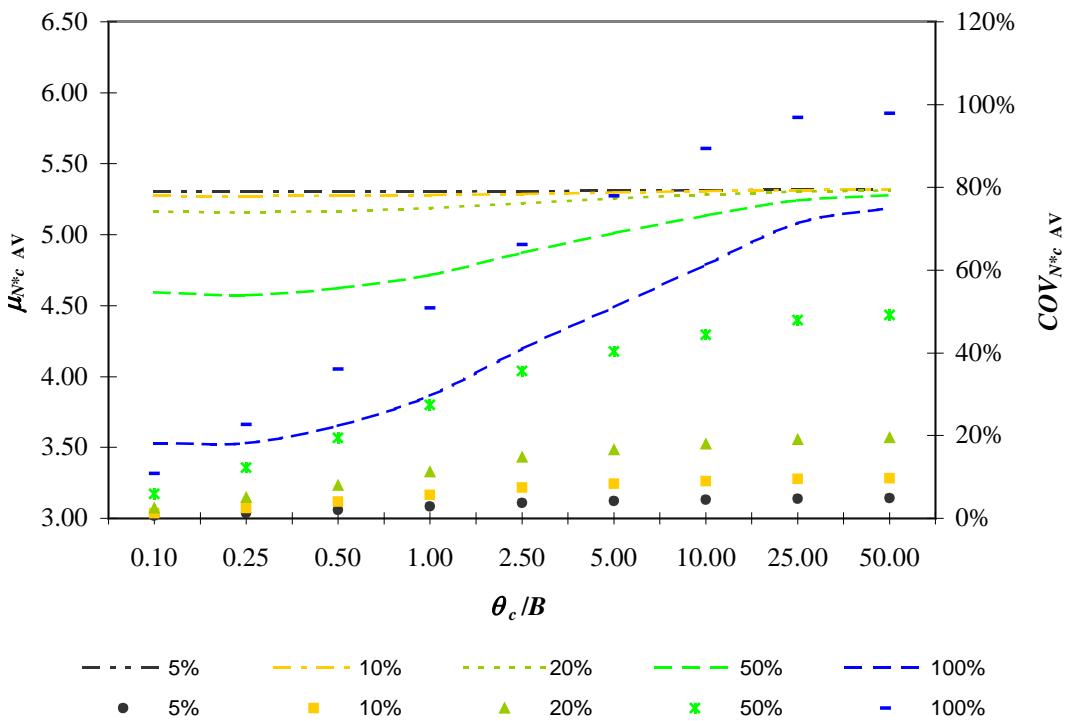


Figure B.48 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_0.05_0.5 case (where $\mu_{c1}/\mu_{c2} = 0.05$ and $H/B = 0.5$).

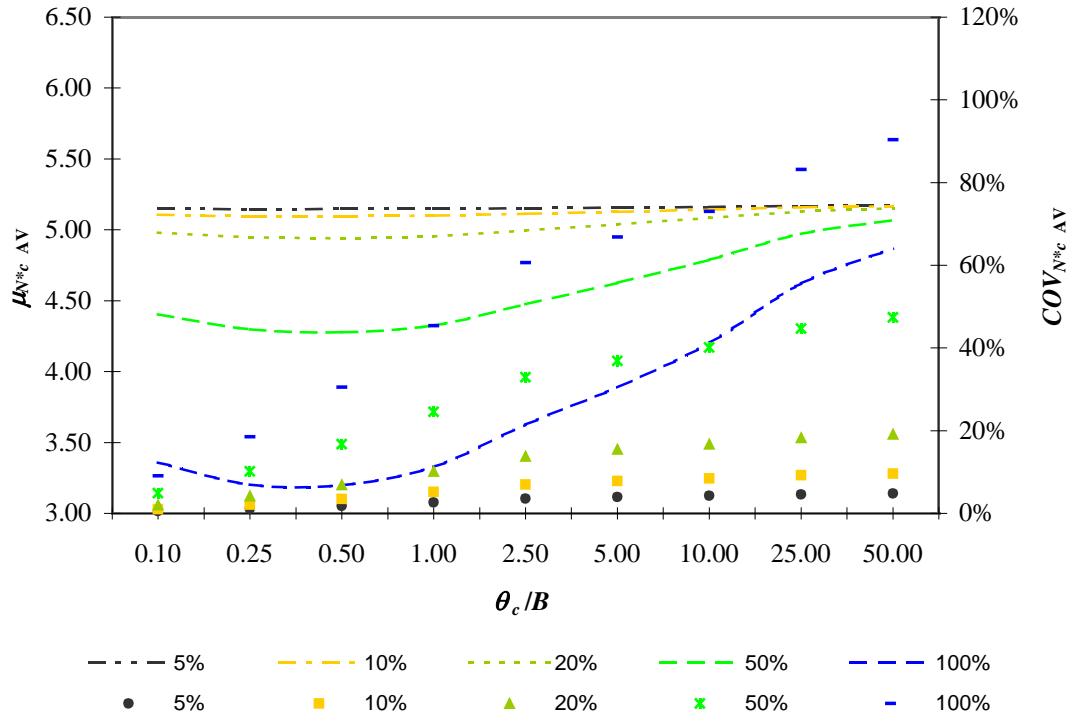


Figure B.49 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_0.05_1.0 case (where $\mu_{c1}/\mu_{c2} = 0.05$ and $H/B = 1.0$).

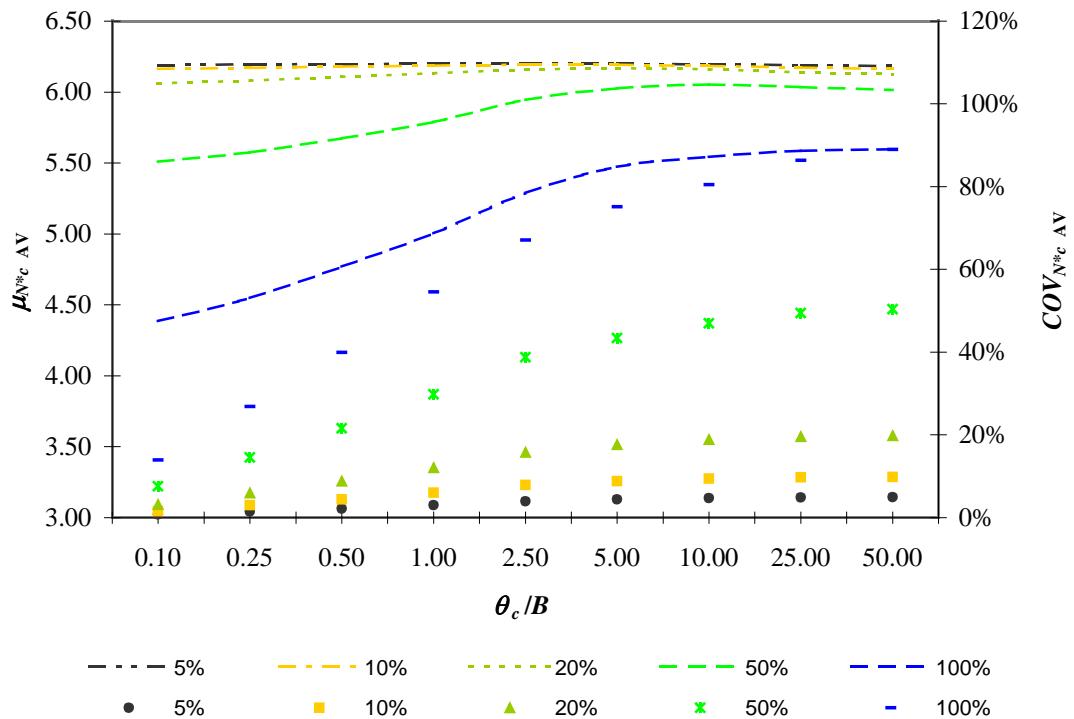


Figure B.50 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_0.1_0.25 case (where $\mu_{c1}/\mu_{c2} = 0.1$ and $H/B = 0.25$).

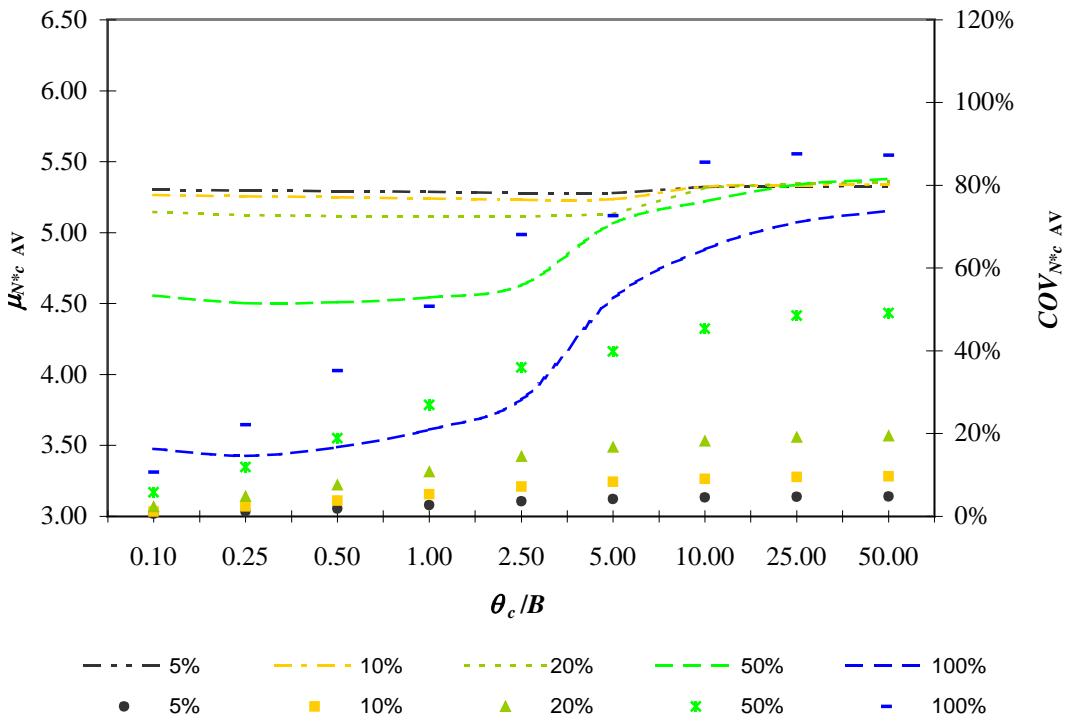


Figure B.51 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_0.1_0.5 case (where $\mu_{c1}/\mu_{c2} = 0.1$ and $H/B = 0.5$).

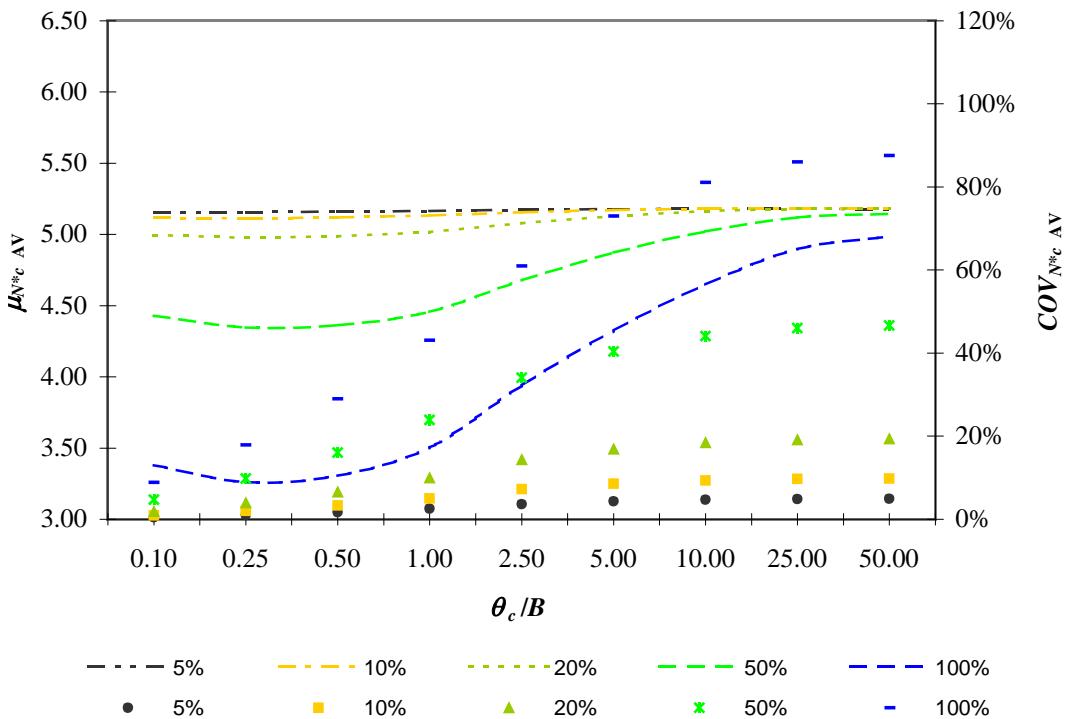
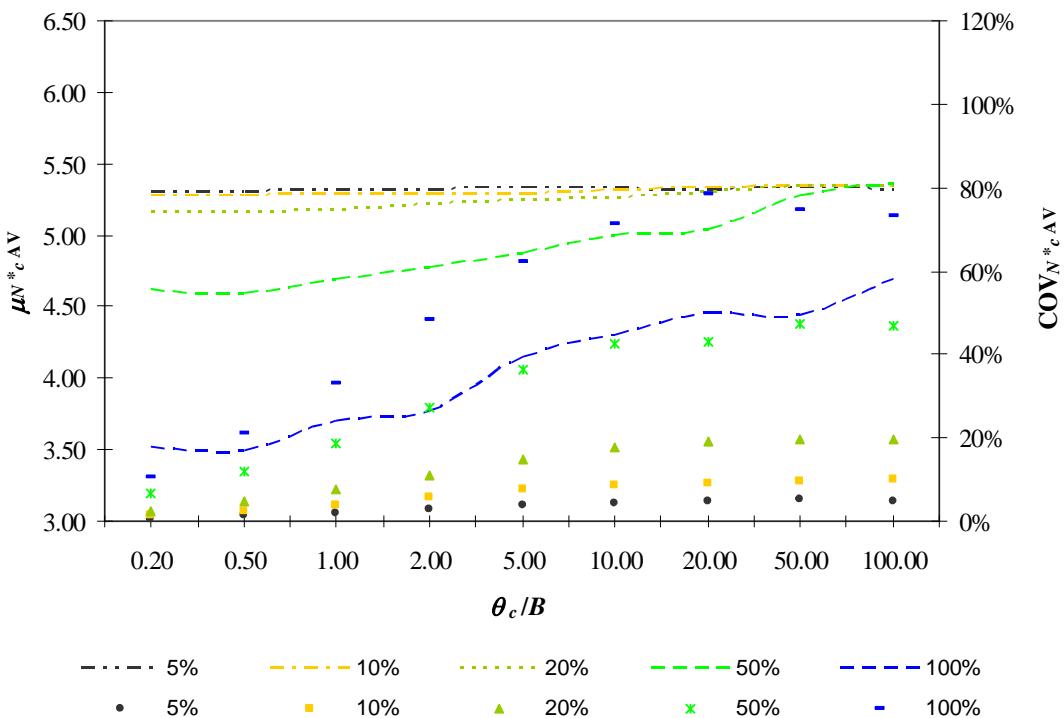
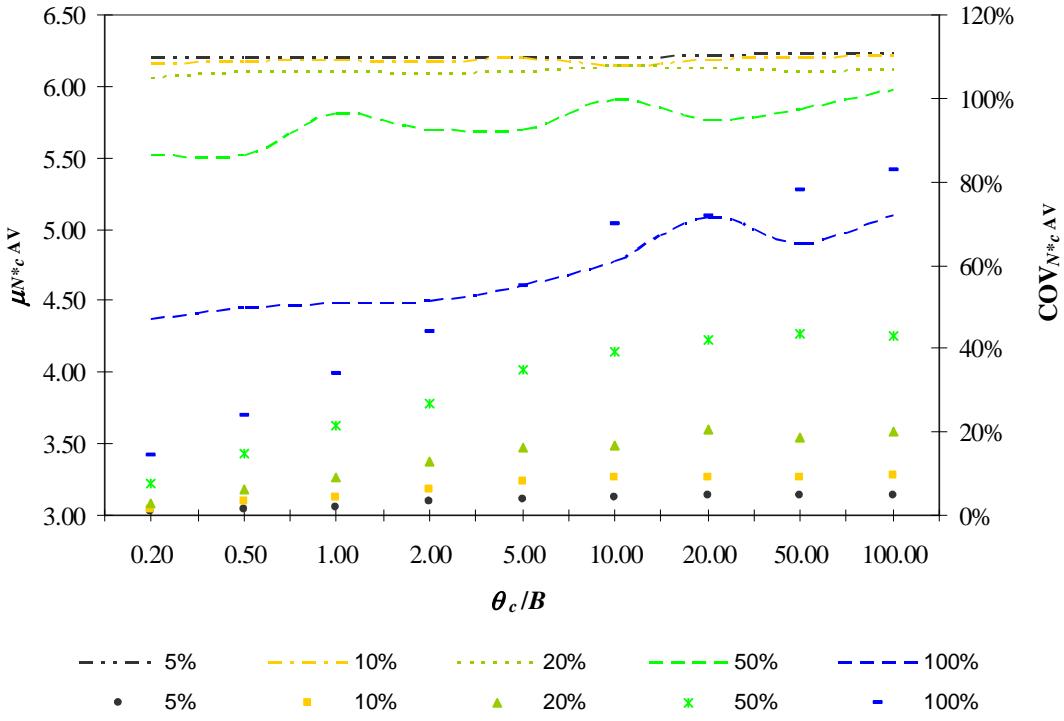


Figure B.52 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_0.1_1.0 case (where $\mu_{c1}/\mu_{c2} = 0.1$ and $H/B = 1.0$).



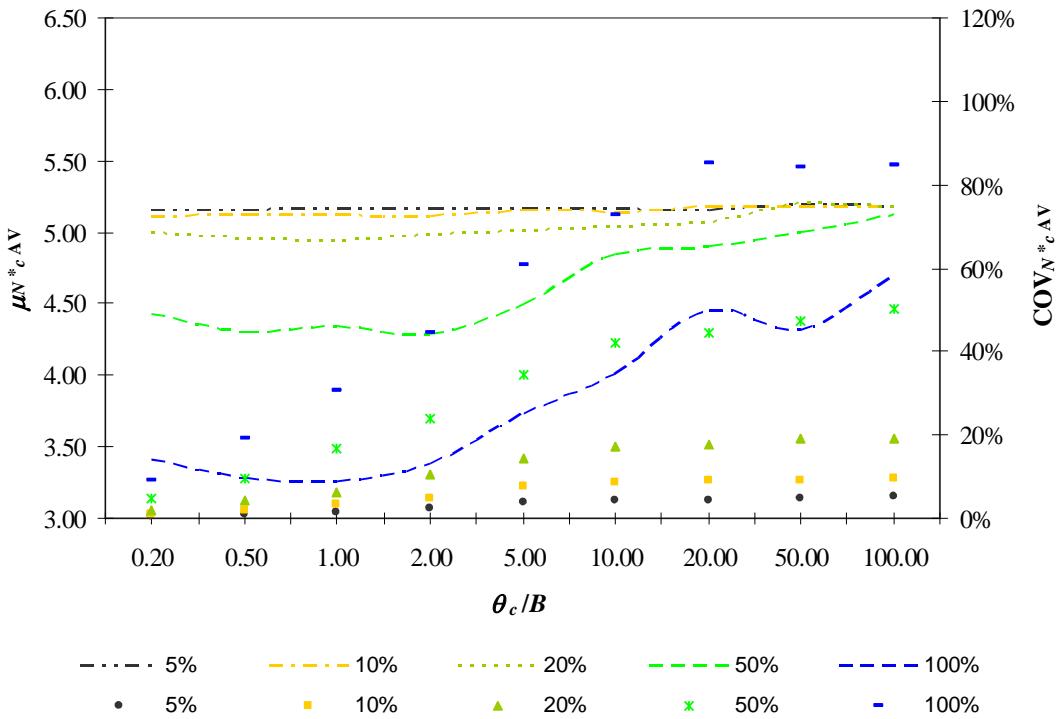


Figure B.55 The variation of $\mu_{N^*c} \text{AV}$ and $\text{COV}_{N^*c} \text{AV}$ with respect to COV_c and θ_c/B for COHESIVE_0.333_1.0 case (where $c_{u1}/c_{u2} = 0.333$ and $H/B = 1.0$).

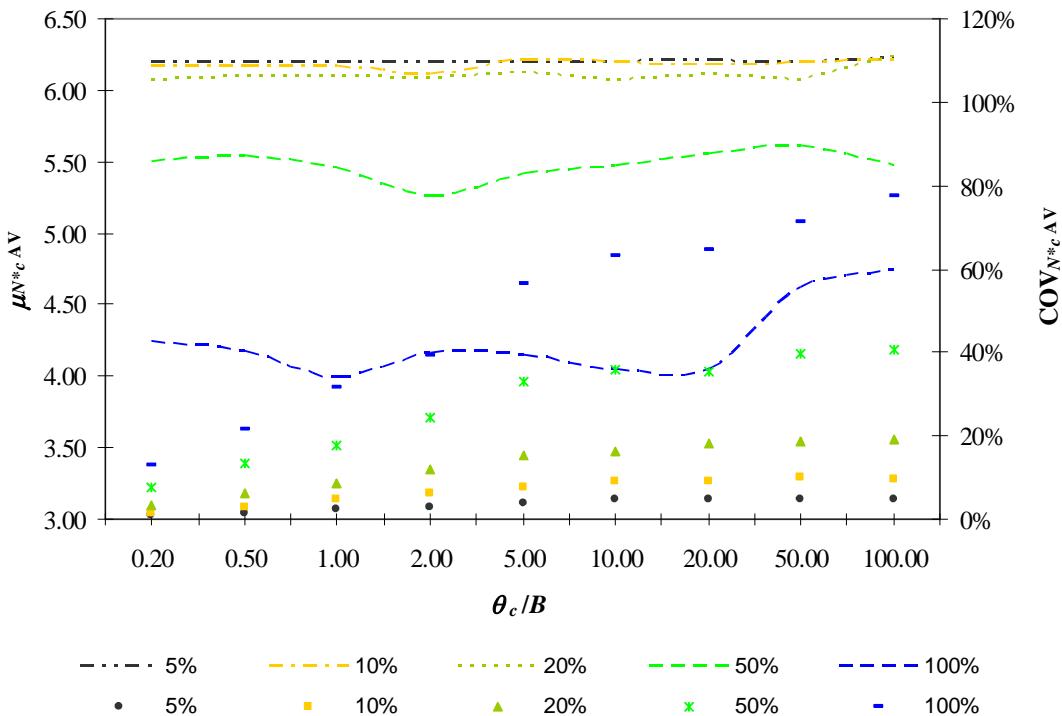


Figure B.56 The variation of $\mu_{N^*c} \text{AV}$ and $\text{COV}_{N^*c} \text{AV}$ with respect to COV_c and θ_c/B for COHESIVE_0.50_0.25 case (where $c_{u1}/c_{u2} = 0.50$ and $H/B = 0.25$).

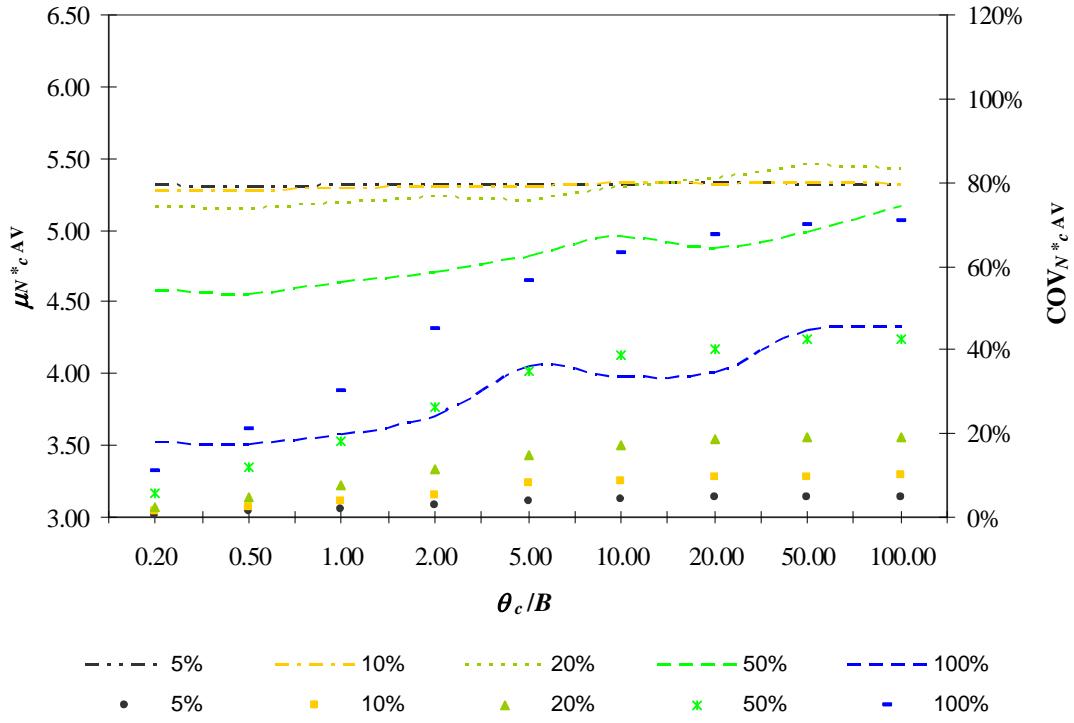


Figure B.57 The variation of $\mu_{N^*c}^{AV}$ and $COV_{N^*c}^{AV}$ with respect to COV_c and θ_c/B for COHESIVE_0.50_0.5 case (where $c_{u1}/c_{u2} = 0.50$ and $H/B = 0.5$).

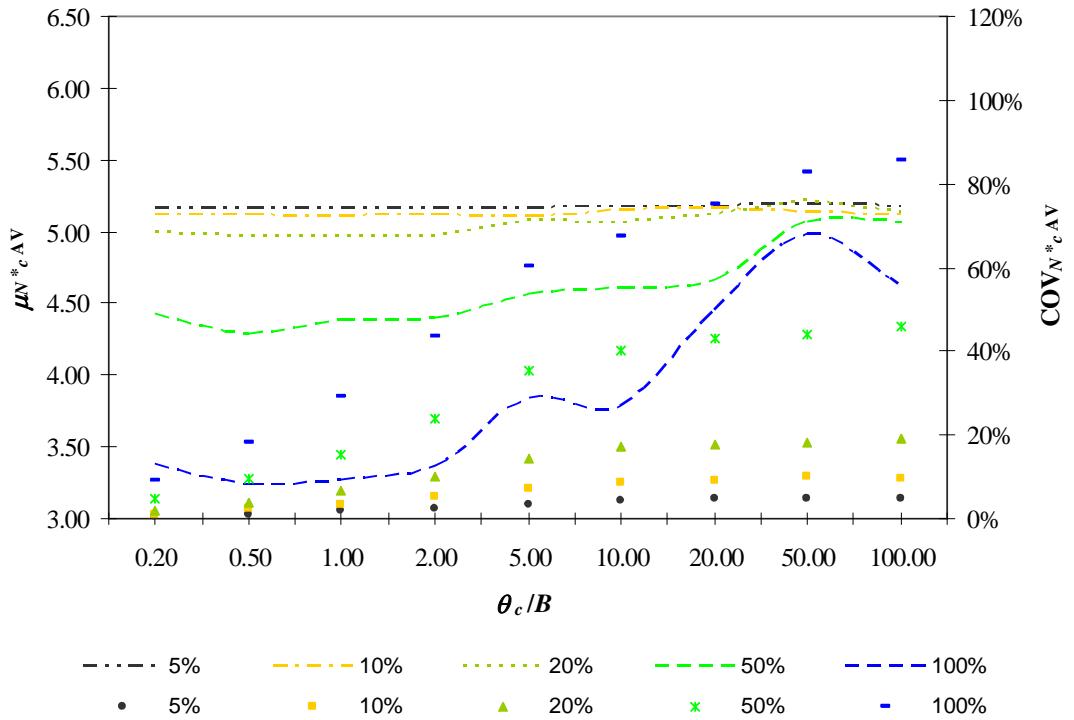


Figure B.58 The variation of $\mu_{N^*c}^{AV}$ and $COV_{N^*c}^{AV}$ with respect to COV_c and θ_c/B for COHESIVE_0.50_1.0 case (where $c_{u1}/c_{u2} = 0.50$ and $H/B = 1.0$).

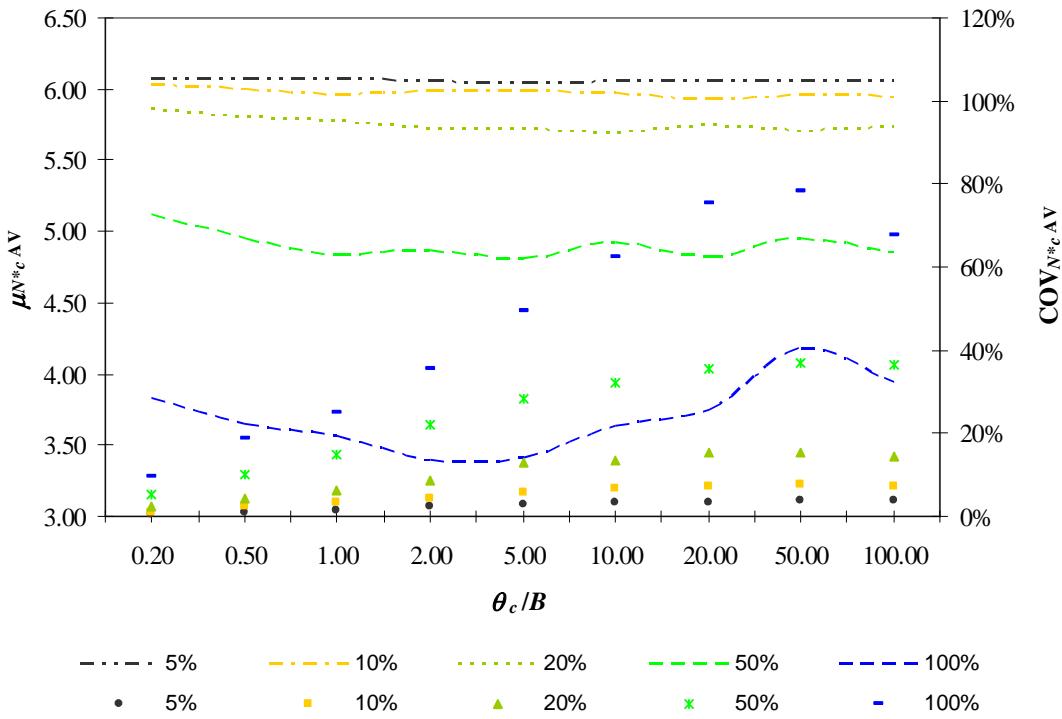


Figure B.59 The variation of $\mu_{N^*c \text{ AV}}$ and $COV_{N^*c \text{ AV}}$ with respect to COV_c and θ_c/B for COHESIVE_0.75_0.25 case (where $c_{u1}/c_{u2} = 0.75$ and $H/B = 0.25$).

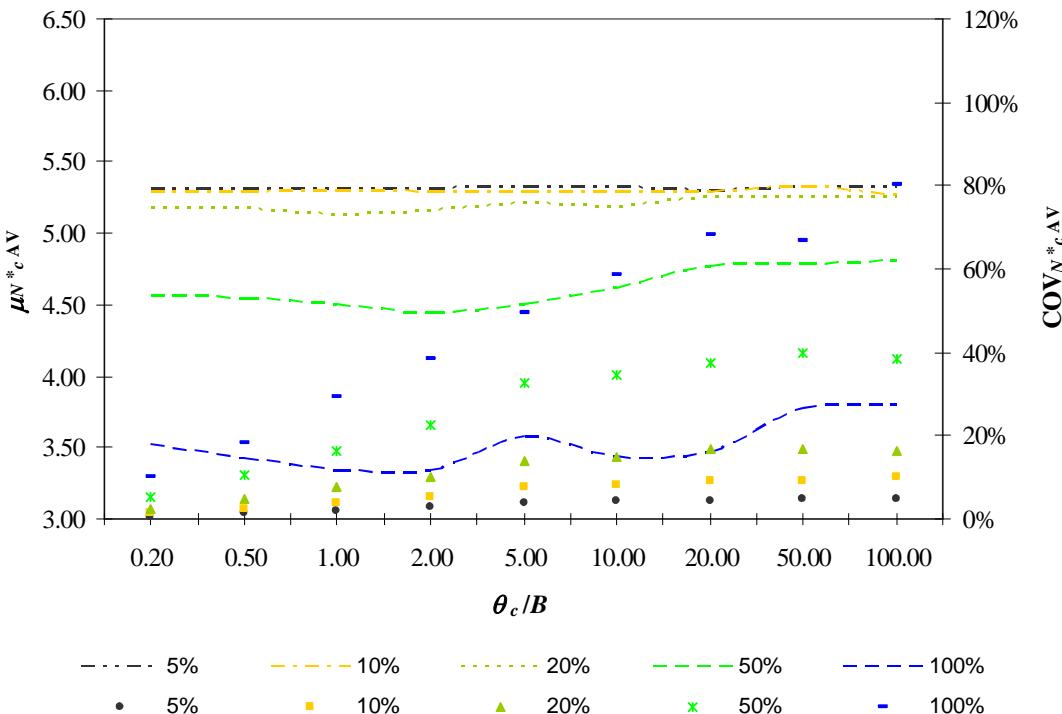


Figure B.60 The variation of $\mu_{N^*c \text{ AV}}$ and $COV_{N^*c \text{ AV}}$ with respect to COV_c and θ_c/B for COHESIVE_0.75_0.5 case (where $c_{u1}/c_{u2} = 0.75$ and $H/B = 0.5$).

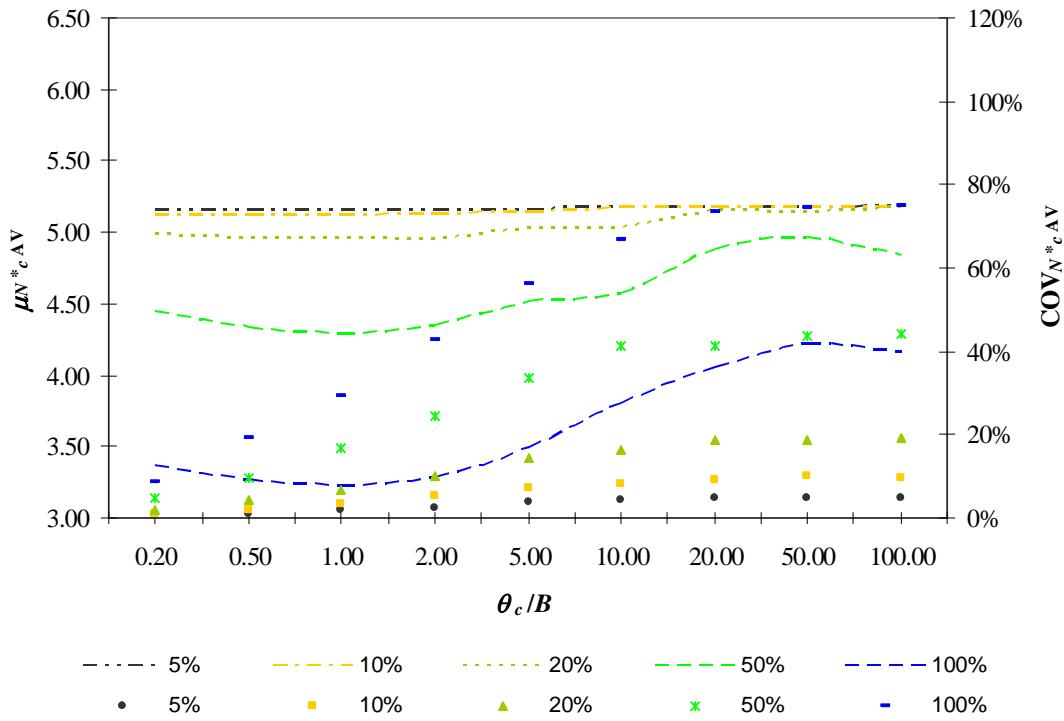


Figure B.61 The variation of $\mu_{N^*c} \text{ AV}$ and $\text{COV}_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_0.75_1.0 case (where $c_{u1}/c_{u2} = 0.75$ and $H/B = 1.0$).

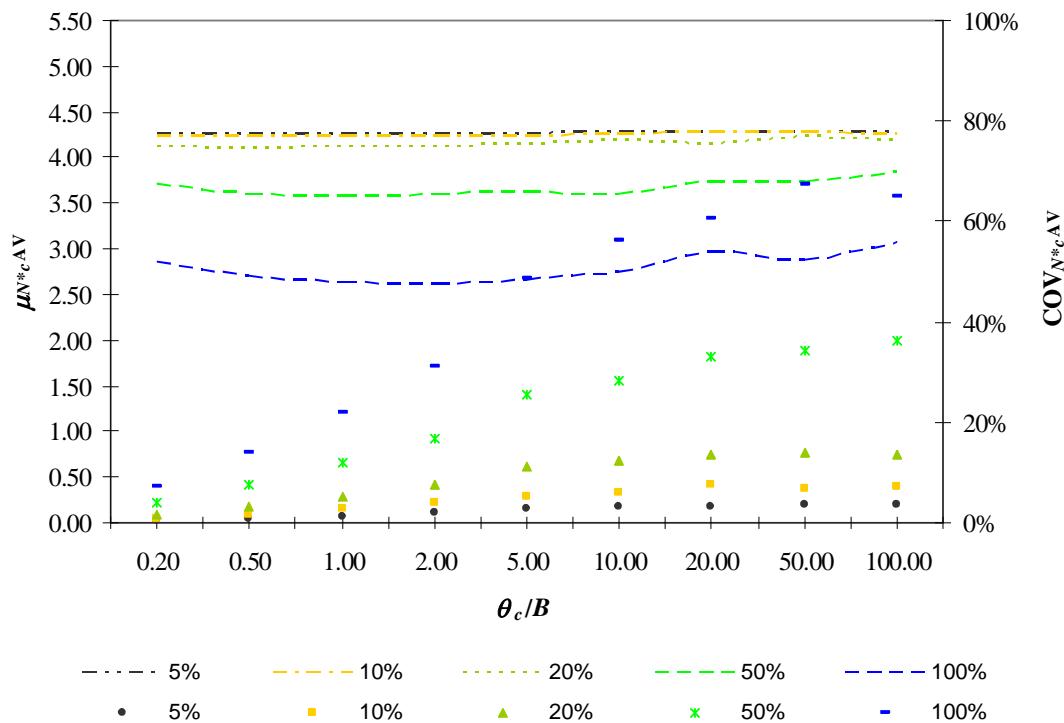


Figure B.62 The variation of $\mu_{N^*c} \text{ AV}$ and $\text{COV}_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_1.333_0.25 case (where $c_{u1}/c_{u2} = 1.333$ and $H/B = 0.25$).

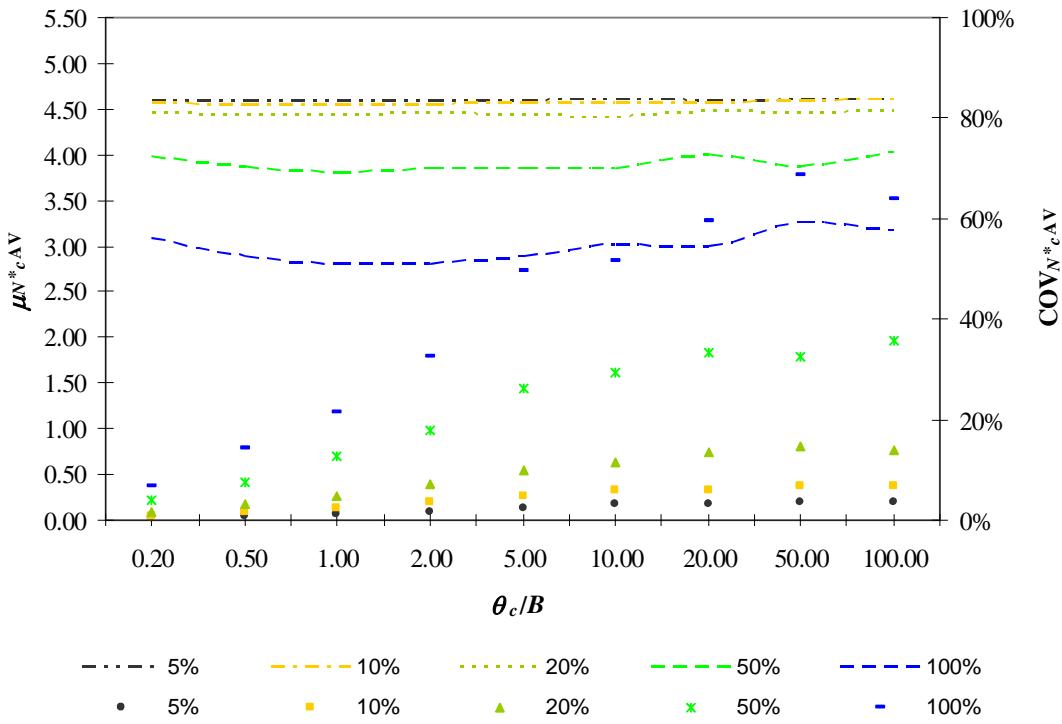


Figure B.63 The variation of $\mu_{N^*_c \text{ AV}}$ and $COV_{N^*_c \text{ AV}}$ with respect to COV_c and θ_c/B for COHESIVE_1.333_0.5 case (where $c_{u1}/c_{u2} = 1.333$ and $H/B = 0.5$).

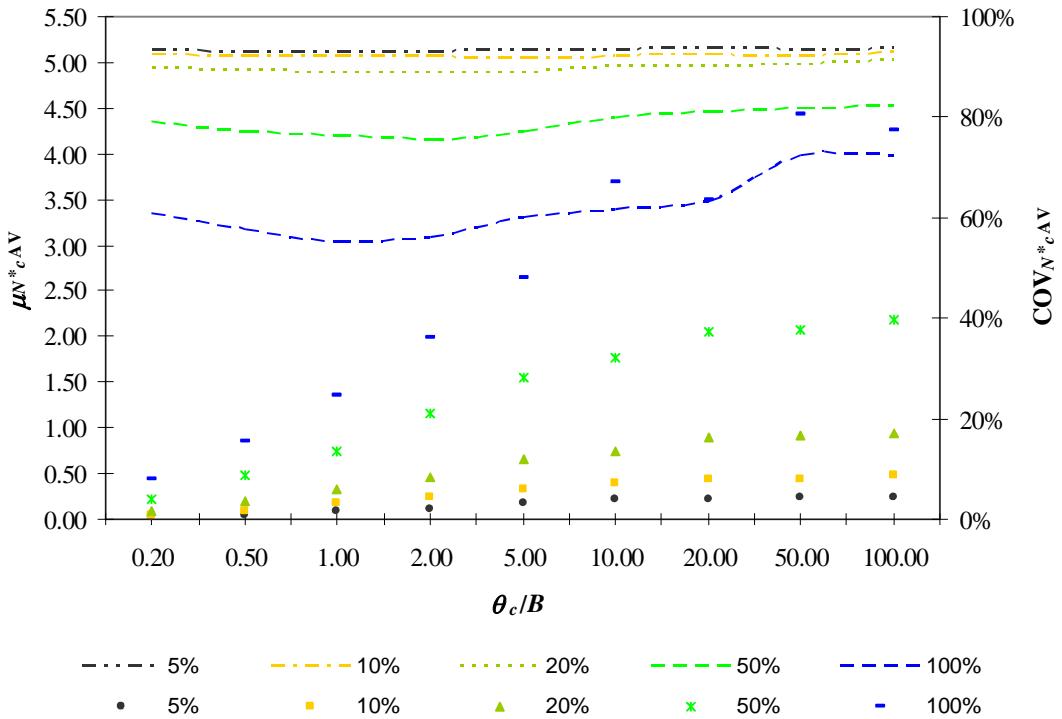


Figure B.64 The variation of $\mu_{N^*_c \text{ AV}}$ and $COV_{N^*_c \text{ AV}}$ with respect to COV_c and θ_c/B for COHESIVE_1.333_1.0 case (where $c_{u1}/c_{u2} = 1.333$ and $H/B = 1.0$).

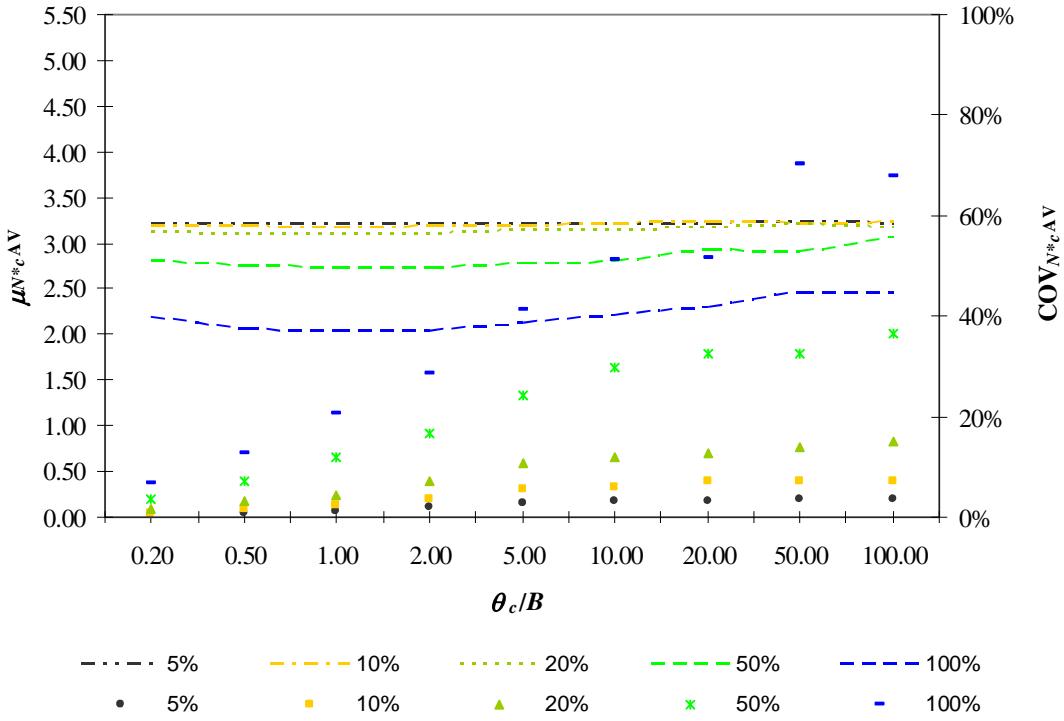


Figure B.65 The variation of $\mu_{N^*c}^{AV}$ and $COV_{N^*c}^{AV}$ with respect to COV_c and θ_c/B for COHESIVE_2.0_0.25 case (where $c_{u1}/c_{u2} = 2.00$ and $H/B = 0.25$).

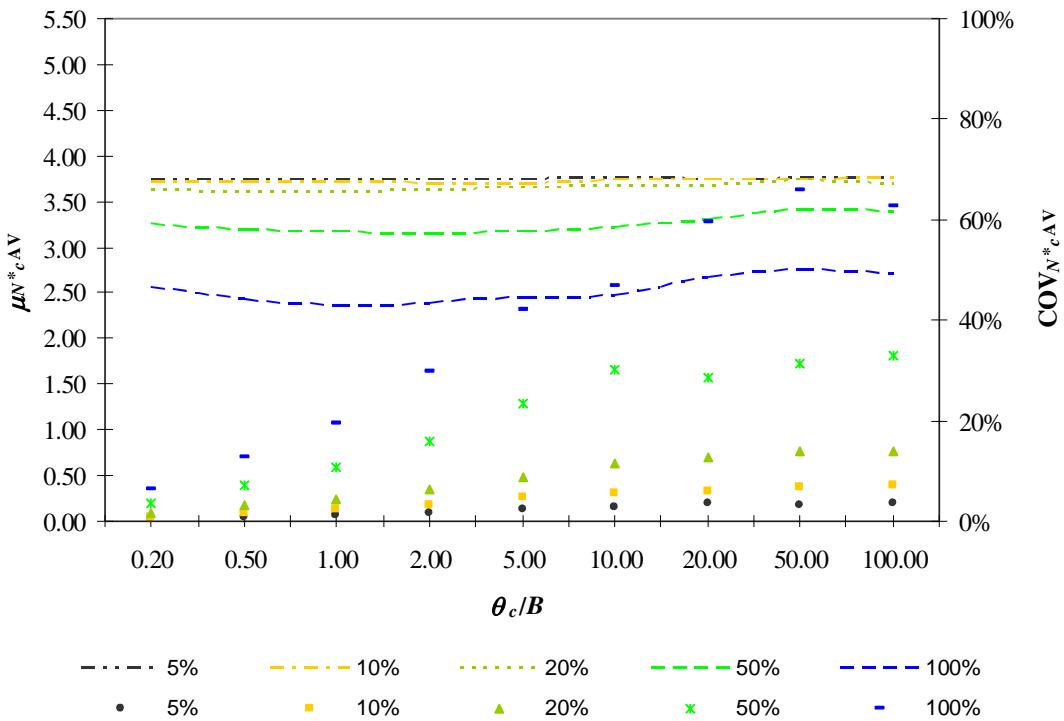


Figure B.66 The variation of $\mu_{N^*c}^{AV}$ and $COV_{N^*c}^{AV}$ with respect to COV_c and θ_c/B for COHESIVE_2.0_0.5 case (where $c_{u1}/c_{u2} = 2.00$ and $H/B = 0.5$).

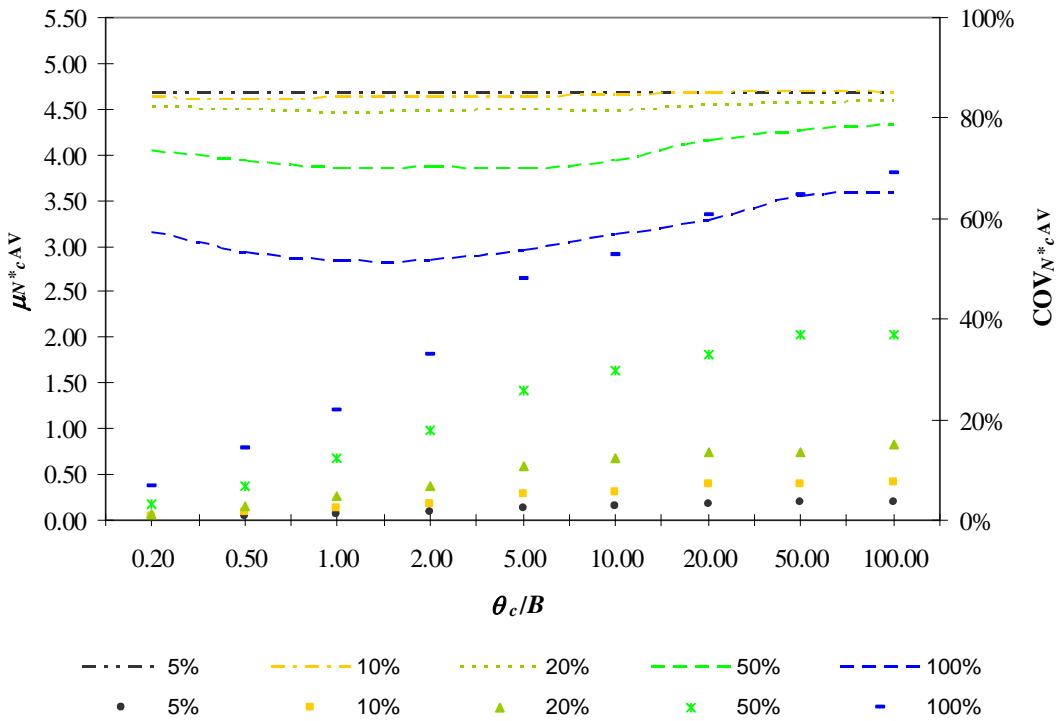


Figure B.67 The variation of $\mu_{N^*c\text{ AV}}$ and $COV_{N^*c\text{ AV}}$ with respect to COV_c and θ_c/B for COHESIVE_2.0_1.0 case (where $c_{u1}/c_{u2} = 2.00$ and $H/B = 1.0$).

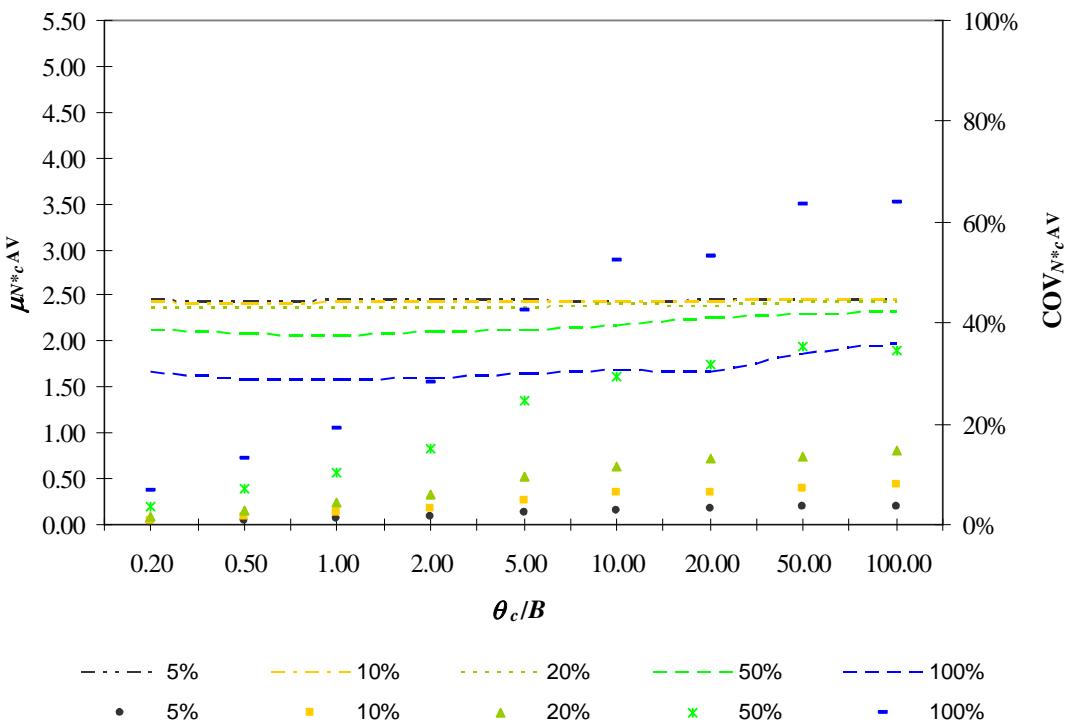


Figure B.68 The variation of $\mu_{N^*c\text{ AV}}$ and $COV_{N^*c\text{ AV}}$ with respect to COV_c and θ_c/B for COHESIVE_3.0_0.25 case (where $c_{u1}/c_{u2} = 3.00$ and $H/B = 0.25$).

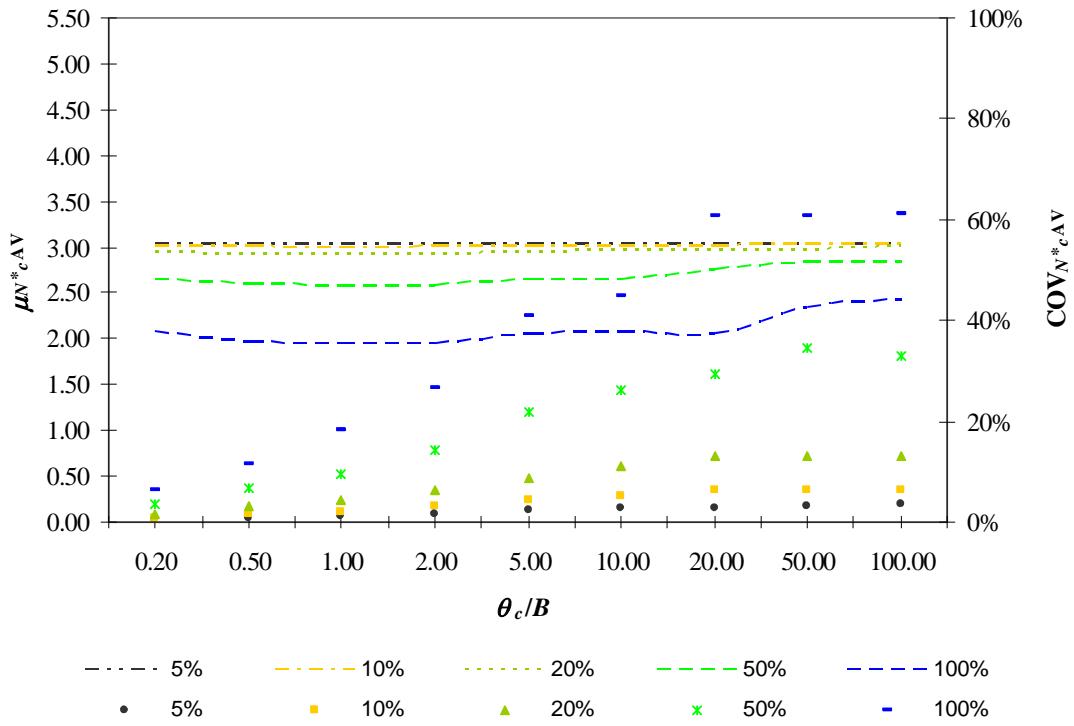


Figure B.69 The variation of $\mu_{N^*c} \text{ AV}$ and $\text{COV}_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_3.0_0.5 case (where $c_{u1}/c_{u2} = 3.00$ and $H/B = 0.5$).

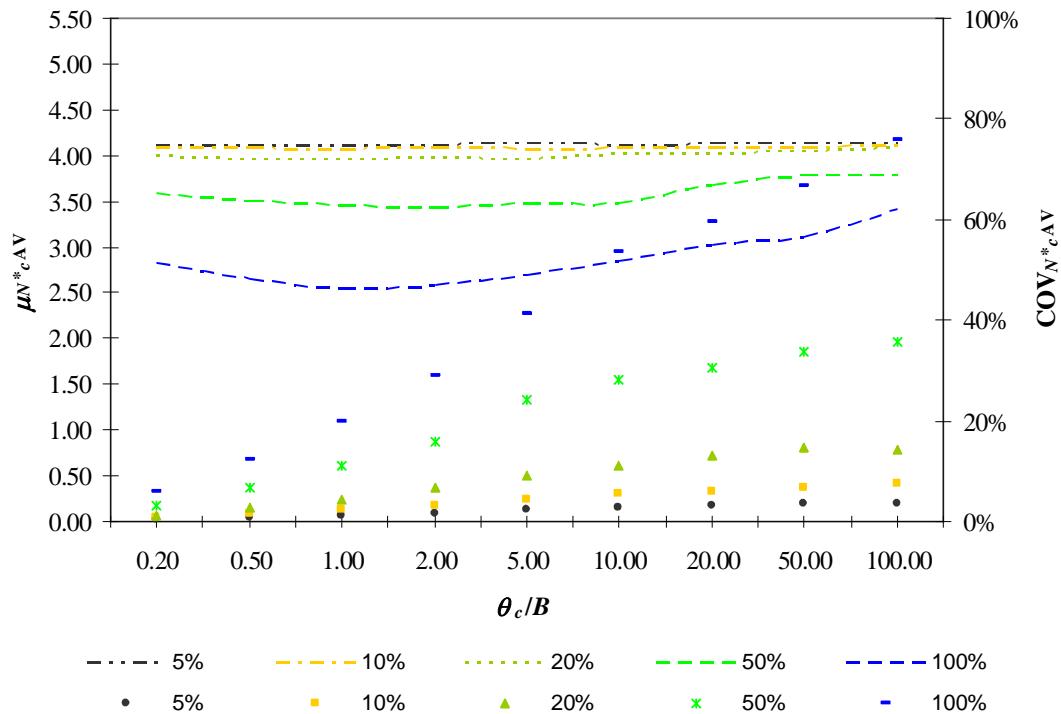


Figure B.70 The variation of $\mu_{N^*c} \text{ AV}$ and $\text{COV}_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_3.0_1.0 case (where $c_{u1}/c_{u2} = 3.00$ and $H/B = 1.0$).

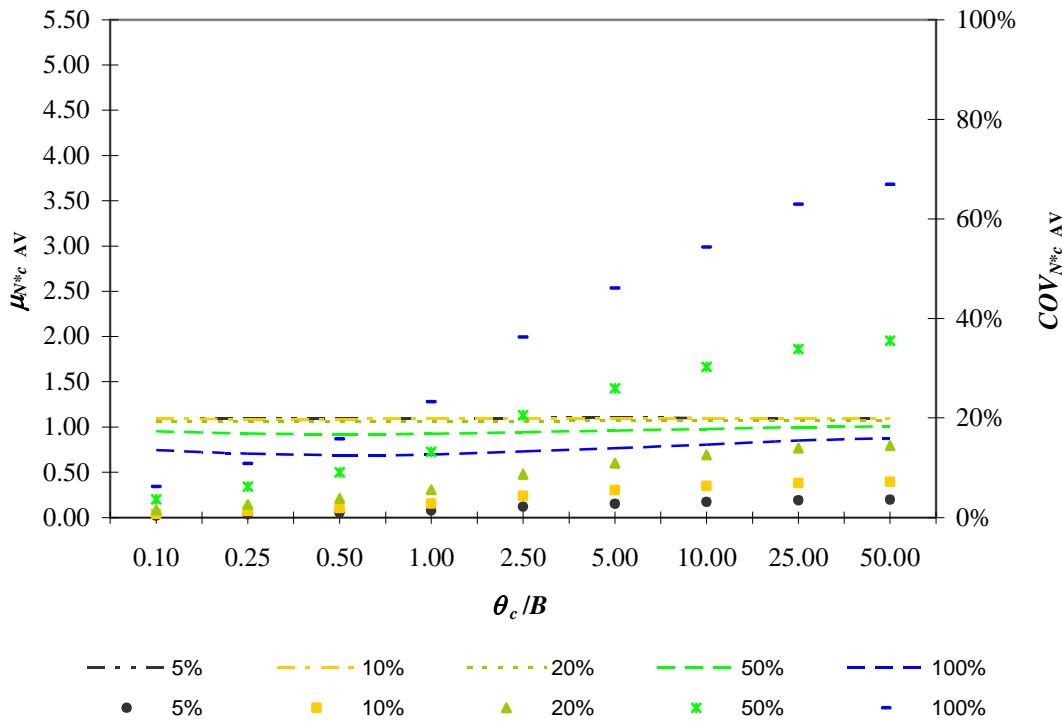


Figure B.71 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_10.0_0.25 case (where $c_{u1}/c_{u2} = 10.0$ and $H/B = 0.25$).

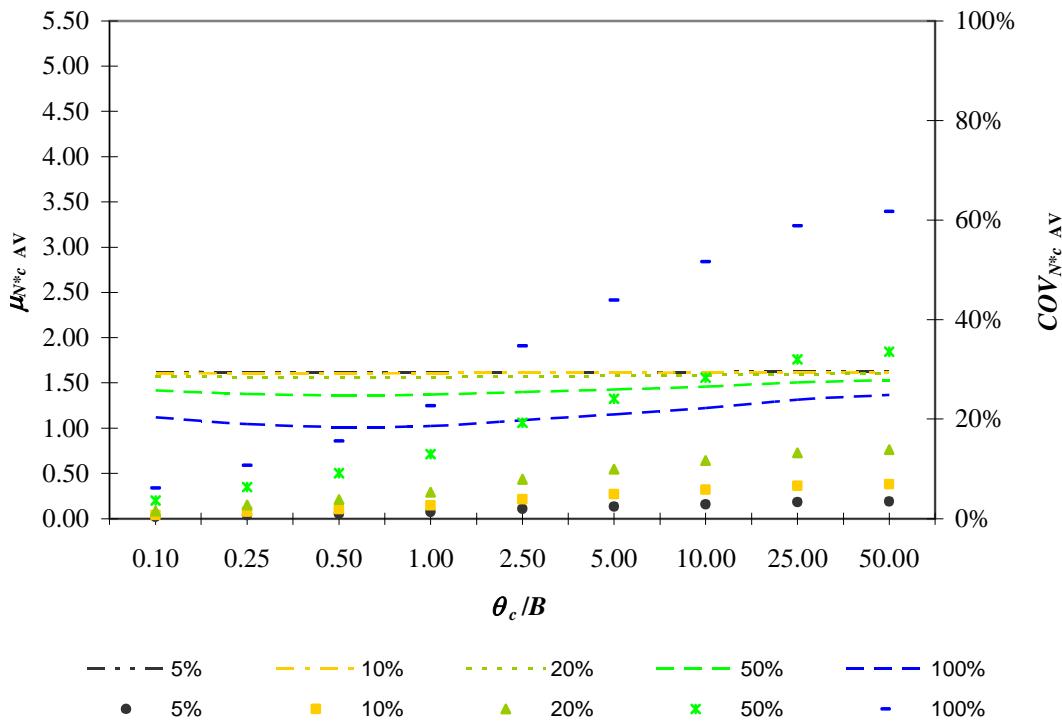


Figure B.72 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_10.0_0.5 case (where $c_{u1}/c_{u2} = 10.0$ and $H/B = 0.5$).

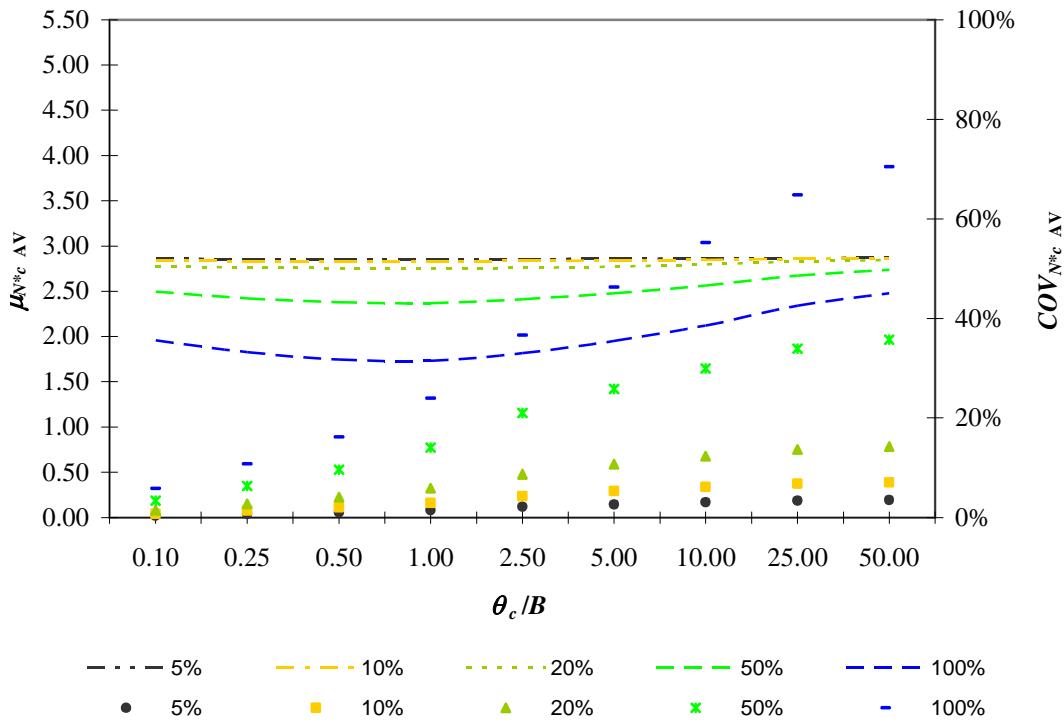


Figure B.73 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_10.0_1.0 case (where $c_{u1}/c_{u2} = 10.0$ and $H/B = 1.0$).

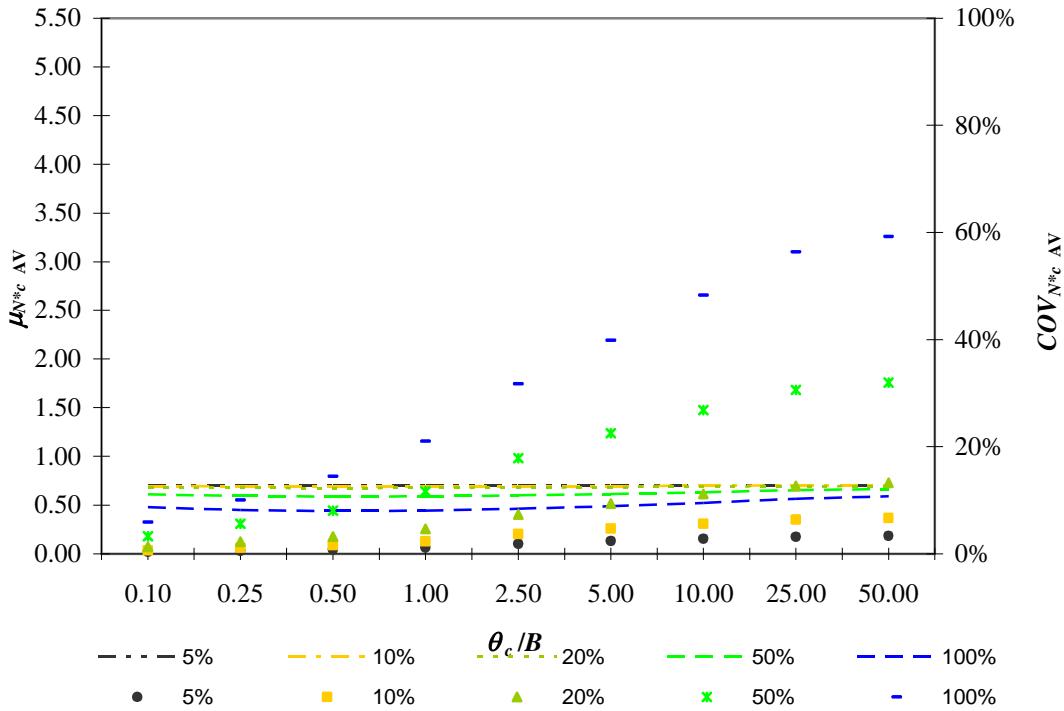


Figure B.74 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_20.0_0.25 case (where $c_{u1}/c_{u2} = 20.0$ and $H/B = 0.25$).

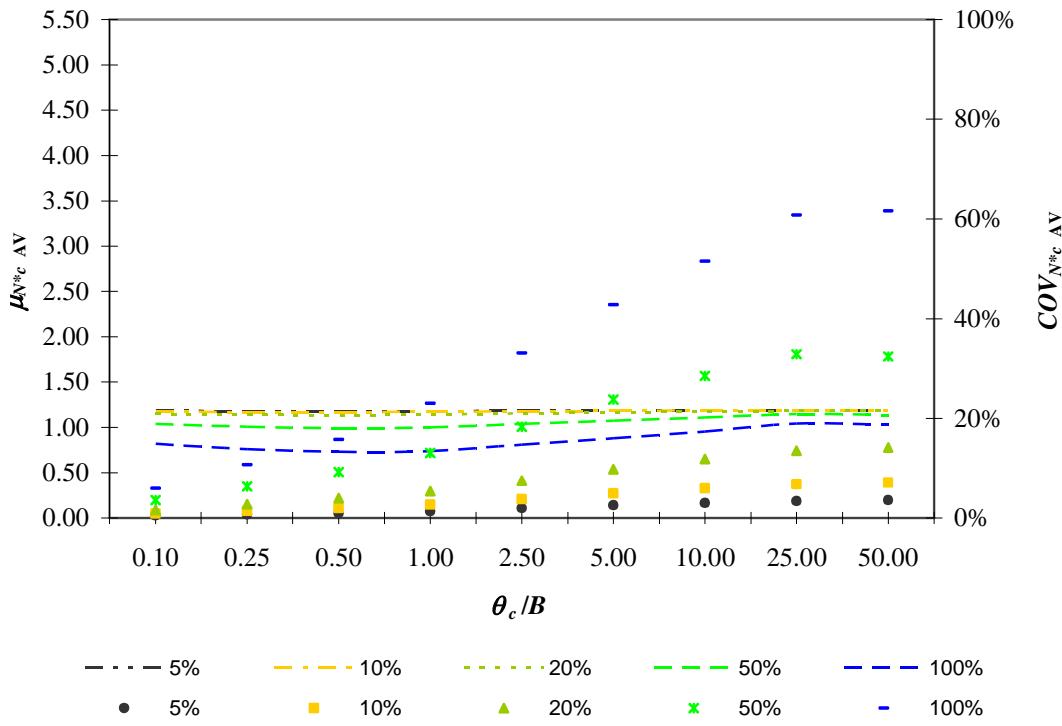


Figure B.75 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_20.0_0.5 case (where $c_{u1}/c_{u2} = 20.0$ and $H/B = 0.5$).

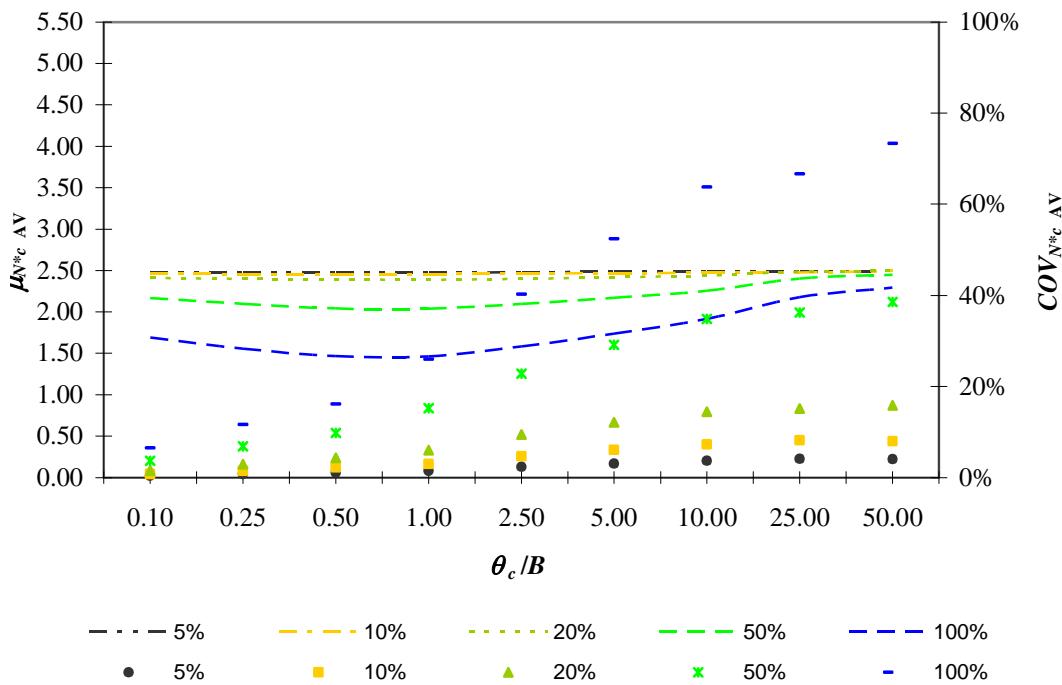


Figure B.76 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_20.0_1.0 case (where $c_{u1}/c_{u2} = 20.0$ and $H/B = 1.0$).

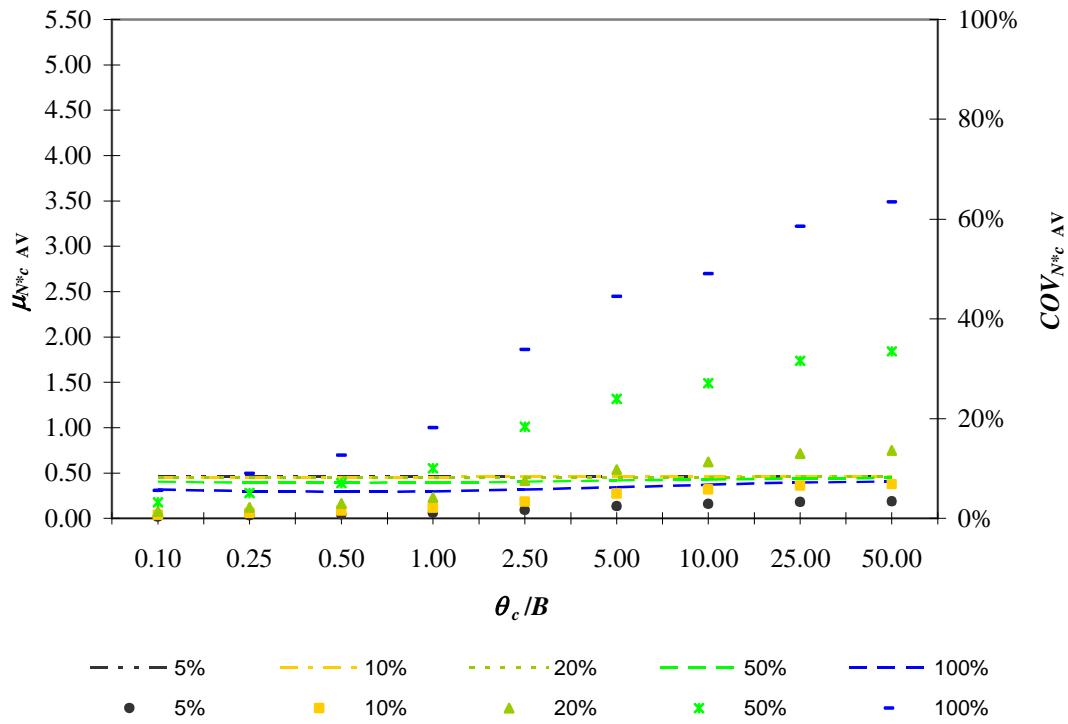


Figure B.77 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_40.0_0.25 case (where $c_{u1}/c_{u2} = 40.0$ and $H/B = 0.25$).

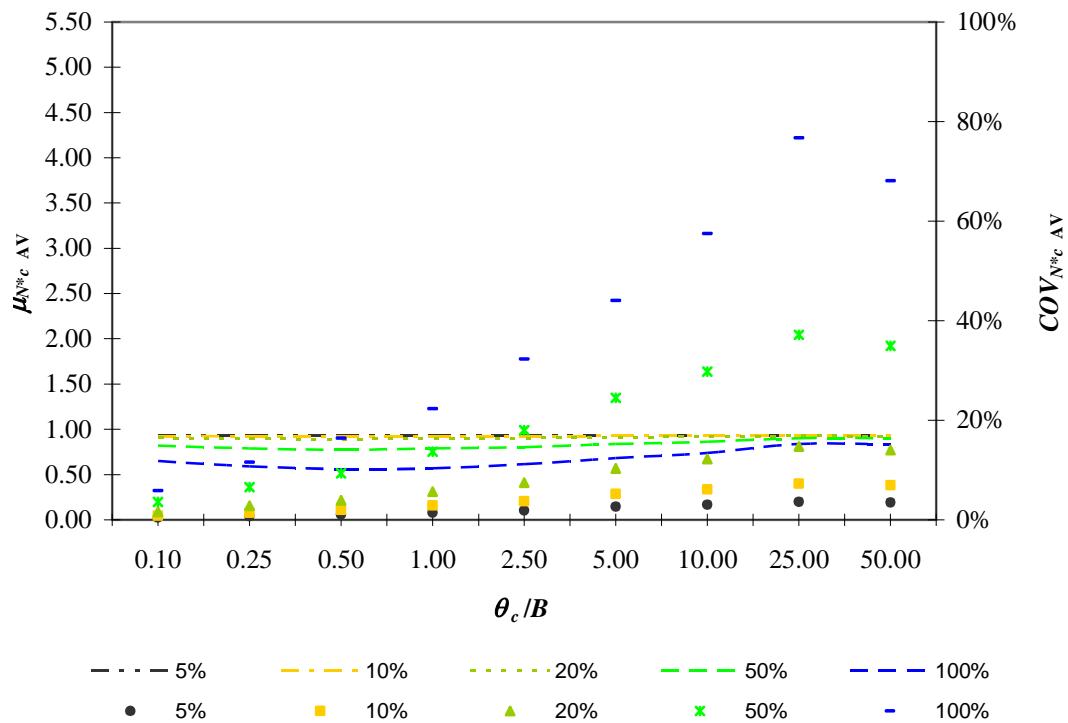


Figure B.78 The variation of $\mu_{N^*c} \text{ AV}$ and $COV_{N^*c} \text{ AV}$ with respect to COV_c and θ_c/B for COHESIVE_40.0_0.5 case (where $c_{u1}/c_{u2} = 40.0$ and $H/B = 0.5$).

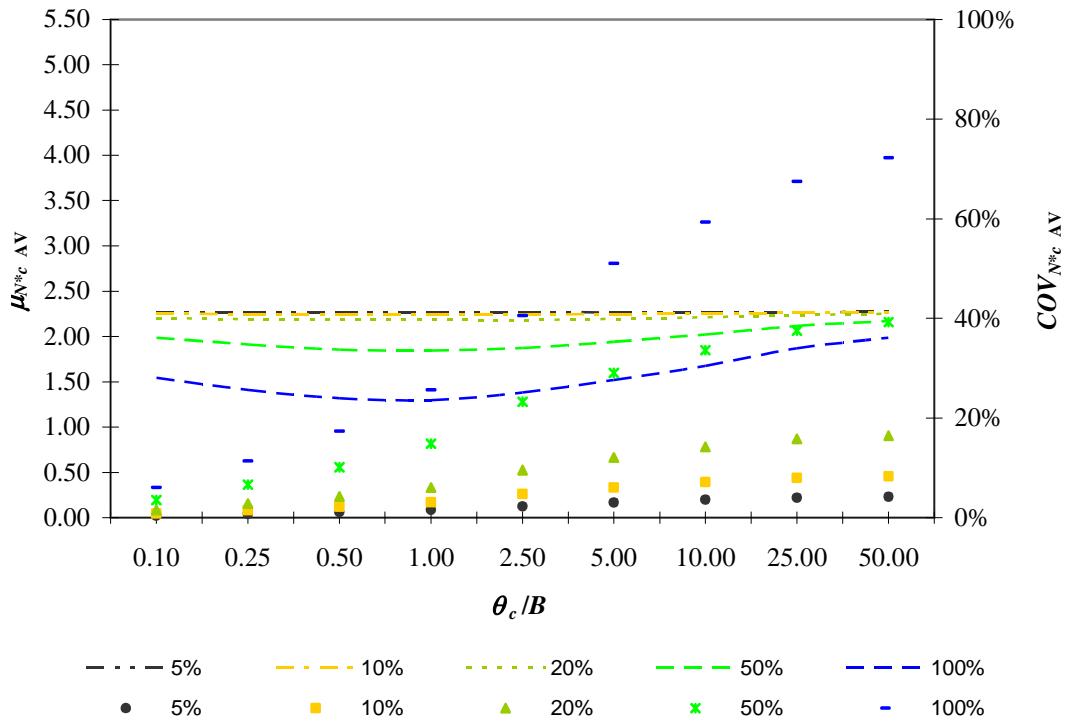


Figure B.79 The variation of $\mu_{N^*c \text{ AV}}$ and $COV_{N^*c \text{ AV}}$ with respect to COV_c and θ_c/B for COHESIVE_40.0_1.0 case (where $c_{u1}/c_{u2} = 40.0$ and $H/B = 1.0$).

APPENDIX C

Table C.1 The results of the ANN input and output statistics.

Model Variable and Data Sets	Mean	Standard Deviation	Maximum	Minimum	Range
Soil cohesion of layer 1 (c_1)					
Training set	5.47	2.60	10.00	1.00	9.00
Testing set	5.29	2.43	9.99	1.12	8.87
Validation set	5.46	2.63	9.99	1.01	8.98
Soil cohesion of layer 2 (c_2)					
Training set	5.42	2.54	10.00	1.00	9.00
Testing set	5.53	2.59	9.99	1.02	8.97
Validation set	5.29	2.63	9.97	1.01	8.96
Soil cohesion of layer 3 (c_3)					
Training set	5.48	2.63	10.00	1.00	9.00
Testing set	5.51	2.64	9.99	1.02	8.97
Validation set	5.74	2.57	9.98	1.01	8.97
Soil cohesion of layer 4 (c_4)					
Training set	5.54	2.63	10.00	1.00	9.00
Testing set	5.27	2.59	9.99	1.01	8.98
Validation set	5.38	2.62	9.99	1.01	8.98
Soil cohesion of layer 5 (c_5)					
Training set	5.51	2.55	10.00	1.00	9.00
Testing set	5.38	2.58	9.97	1.01	8.96
Validation set	5.42	2.59	10.00	1.00	9.00
Soil cohesion of layer 6 (c_6)					
Training set	5.70	2.58	9.99	1.01	8.98
Testing set	5.52	2.52	9.99	1.02	8.97
Validation set	5.42	2.70	9.99	1.02	8.97
Soil cohesion of layer 7 (c_7)					
Training set	5.35	2.53	10.00	1.00	9.00
Testing set	5.64	2.66	9.96	1.01	8.95
Validation set	5.41	2.59	10.00	1.02	9.98
Soil cohesion of layer 8 (c_8)					
Training set	5.67	2.62	10.00	1.00	9.00
Testing set	5.31	2.49	9.99	1.03	8.96
Validation set	5.47	2.59	9.99	1.02	8.97
Soil cohesion of layer 9 (c_9)					
Training set	5.58	2.60	10.00	1.00	9.00
Testing set	5.41	2.54	9.97	1.02	8.95
Validation set	5.52	2.63	9.99	1.00	8.99

Table C.1 The results of the ANN input and output statistics. (*Continued*)

Model Variable and Data Sets	Mean	Standard Deviation	Maximum	Minimum	Range
Soil cohesion of layer 10 (c_{10})					
Training set	5.41	2.64	9.98	1.01	8.97
Testing set	5.60	2.74	9.97	1.05	8.92
Validation set	5.49	2.62	9.97	1.01	8.96
Friction angle of layer 1 (ϕ_1)					
Training set	12.57	4.32	20.00	5.01	14.99
Testing set	12.68	4.24	19.99	5.02	14.97
Validation set	12.62	4.29	19.99	5.01	14.98
Friction angle of layer 2 (ϕ_2)					
Training set	12.48	4.34	20.00	5.01	14.99
Testing set	12.45	4.33	19.99	5.03	14.96
Validation set	12.23	4.36	19.91	5.04	14.87
Friction angle of layer 3 (ϕ_3)					
Training set	12.40	4.35	19.98	5.00	14.98
Testing set	12.25	4.32	19.93	5.09	14.84
Validation set	12.29	4.46	19.98	5.01	14.97
Friction angle of layer 4 (ϕ_4)					
Training set	12.70	4.41	19.99	5.01	14.98
Testing set	12.56	4.36	19.99	5.15	14.84
Validation set	12.19	4.29	19.98	5.01	14.97
Friction angle of layer 5 (ϕ_5)					
Training set	12.39	4.39	20.00	5.00	15.00
Testing set	12.33	4.25	19.98	5.01	14.97
Validation set	12.50	4.41	19.97	5.02	14.95
Friction angle of layer 6 (ϕ_6)					
Training set	12.83	4.27	19.99	5.00	14.99
Testing set	12.60	4.19	19.99	5.10	14.89
Validation set	12.39	4.33	19.96	5.01	14.95
Friction angle of layer 7 (ϕ_7)					
Training set	12.39	4.38	19.99	5.00	14.99
Testing set	12.45	4.25	19.99	5.09	14.90
Validation set	12.34	4.35	19.97	5.01	14.96
Friction angle of layer 8 (ϕ_8)					
Training set	12.37	4.39	19.99	5.00	14.99
Testing set	12.28	4.33	19.86	5.01	14.85
Validation set	12.48	4.33	19.98	5.04	14.94

Table C.1 The results of the ANN input and output statistics. (*Continued*)

Model Variable and Data Sets	Mean	Standard Deviation	Maximum	Minimum	Range
Friction angle of layer 9 (ϕ_9)					
Training set	12.62	4.35	20.00	5.00	15.00
Testing set	12.42	4.37	19.98	5.00	14.98
Validation set	12.37	4.32	19.94	5.04	14.90
Friction angle of layer 10 (ϕ_{10})					
Training set	12.50	4.36	19.99	5.00	14.99
Testing set	12.88	4.37	20.00	5.03	14.97
Validation set	12.07	4.38	19.94	5.10	14.84
Soil thickness of layer 1 (h_1)					
Training set	0.61	0.29	1.00	0.20	0.80
Testing set	0.58	0.28	1.00	0.20	0.80
Validation set	0.61	0.28	1.00	0.20	0.80
Soil thickness of layer 2 (h_2)					
Training set	0.62	0.28	1.00	0.20	0.80
Testing set	0.60	0.29	1.00	0.20	0.80
Validation set	0.58	0.27	1.00	0.20	0.80
Soil thickness of layer 3 (h_3)					
Training set	0.60	0.29	1.00	0.20	0.80
Testing set	0.61	0.29	1.00	0.20	0.80
Validation set	0.62	0.29	1.00	0.20	0.80
Soil thickness of layer 4 (h_4)					
Training set	0.60	0.28	1.00	0.20	0.80
Testing set	0.61	0.28	1.00	0.20	0.80
Validation set	0.60	0.29	1.00	0.20	0.80
Soil thickness of layer 5 (h_5)					
Training set	0.59	0.28	1.00	0.20	0.80
Testing set	0.58	0.28	1.00	0.20	0.80
Validation set	0.61	0.29	1.00	0.20	0.80
Soil thickness of layer 6 (h_6)					
Training set	0.60	0.28	1.00	0.20	0.80
Testing set	0.61	0.27	1.00	0.20	0.80
Validation set	0.60	0.29	1.00	0.20	0.80
Soil thickness of layer 7 (h_7)					
Training set	0.61	0.28	1.00	0.20	0.80
Testing set	0.57	0.28	1.00	0.20	0.80
Validation set	0.61	0.28	1.00	0.20	0.80

Table C.1 The results of the ANN input and output statistics. (*Continued*)

Model Variable and Data Sets	Mean	Standard Deviation	Maximum	Minimum	Range
Soil thickness of layer 8 (h_8)					
Training set	0.59	0.28	1.00	0.20	0.80
Testing set	0.59	0.28	1.00	0.20	0.80
Validation set	0.59	0.29	1.00	0.20	0.80
Soil thickness of layer 9 (h_9)					
Training set	0.60	0.29	1.00	0.20	0.80
Testing set	0.61	0.28	1.00	0.20	0.80
Validation set	0.60	0.29	1.00	0.20	0.80
Footing width (B)					
Training set	2.50	0.93	4.00	1.00	3.00
Testing set	2.51	0.93	4.00	1.00	3.00
Validation set	2.51	0.92	4.00	1.00	3.00
Bearing capacity of strip footing (q_u) for $c-\phi$ case					
Training set	39.77	14.49	93.31	7.26	86.05
Testing set	39.60	14.19	87.94	11.67	76.27
Validation set	39.28	14.51	80.19	9.04	71.15
\tilde{N}_c					
Training set	0.858	0.313	2.260	0.174	2.085
Testing set	0.868	0.304	2.106	0.216	1.890
Validation set	0.862	0.321	2.175	0.255	1.920
$\tilde{N}_{c-\phi}$					
Training set	1.043	0.255	3.035	0.524	2.511
Testing set	1.038	0.243	1.951	0.578	1.373
Validation set	1.018	0.224	1.830	0.561	1.269

Table C.2 The results of null hypothesis tests inputs and outputs.

Model Variable and Data Sets	t-value	Lower Critical value	Upper Critical value	t-test	F-value	Lower Critical value	Upper Critical value	F-test
Soil cohesion of layer 1 (c_1)								
Training	1.042	-1.961	1.961	Accept	0.996	-0.937	0.937	Accept
Testing	-1.607	-1.961	1.961	Accept	1.024	-0.916	0.916	Accept
Validation	-0.218	-1.961	1.961	Accept	0.973	-0.869	0.869	Accept

Table C.2 The results of null hypothesis tests inputs and outputs. (*Continued*)

Model Variable and Data Sets	<i>t</i> -value	Lower Critical value	Upper Critical value	<i>t</i> -test	<i>F</i> -value	Lower Critical value	Upper Critical value	<i>F</i> -test
Soil cohesion of layer 2 (c_2)								
Training	0.868	-1.961	1.961	Accept	0.991	-0.937	0.937	Accept
Testing	-0.338	-1.961	1.961	Accept	1.010	-0.916	0.916	Accept
Validation	-1.294	-1.961	1.961	Accept	0.942	-0.869	0.869	Accept
Soil cohesion of layer 3 (c_3)								
Training	-0.737	-1.961	1.961	Accept	0.993	-0.937	0.937	Accept
Testing	0.970	-1.961	1.961	Accept	1.005	-0.916	0.916	Accept
Validation	0.334	-1.961	1.961	Accept	1.071	-0.869	0.869	Accept
Soil cohesion of layer 4 (c_4)								
Training	0.193	-1.961	1.961	Accept	0.997	-0.937	0.937	Accept
Testing	-0.508	-1.961	1.961	Accept	1.011	-0.916	0.916	Accept
Validation	0.194	-1.961	1.961	Accept	0.990	-0.869	0.869	Accept
Soil cohesion of layer 5 (c_5)								
Training	-0.674	-1.961	1.961	Accept	1.026	-0.937	0.937	Accept
Testing	0.546	-1.961	1.961	Accept	0.984	-0.916	0.916	Accept
Validation	0.650	-1.961	1.961	Accept	0.991	-0.869	0.869	Accept
Soil cohesion of layer 6 (c_6)								
Training	-0.612	-1.961	1.961	Accept	0.996	-0.937	0.937	Accept
Testing	0.846	-1.961	1.961	Accept	1.050	-0.916	0.916	Accept
Validation	0.228	-1.961	1.961	Accept	0.912	-0.869	0.869	Accept
Soil cohesion of layer 7 (c_7)								
Training	0.895	-1.961	1.961	Accept	1.006	-0.937	0.937	Accept
Testing	-1.927	-1.961	1.961	Accept	1.005	-0.916	0.916	Accept
Validation	0.434	-1.961	1.961	Accept	0.969	-0.869	0.869	Accept
Soil cohesion of layer 8 (c_8)								
Training	0.239	-1.961	1.961	Accept	1.007	-0.937	0.937	Accept
Testing	0.207	-1.961	1.961	Accept	0.978	-0.916	0.916	Accept
Validation	-0.689	-1.961	1.961	Accept	0.967	-0.869	0.869	Accept
Soil cohesion of layer 9 (c_9)								
Training	0.360	-1.961	1.961	Accept	1.009	-0.937	0.937	Accept
Testing	0.440	-1.961	1.961	Accept	0.993	-0.916	0.916	Accept
Validation	-1.168	-1.961	1.961	Accept	0.991	-0.869	0.869	Accept

Table C.2 The results of null hypothesis tests inputs and outputs. (*Continued*)

Model Variable and Data Sets	t-value	Lower Critical value	Upper Critical value	t-test	F-value	Lower Critical value	Upper Critical value	F-test
Soil cohesion of layer 10 (c_{10})								
Training	0.159	-1.961	1.961	Accept	0.990	-0.937	0.937	Accept
Testing	0.634	-1.961	1.961	Accept	1.009	-0.916	0.916	Accept
Validation	-1.015	-1.961	1.961	Accept	1.034	-0.869	0.869	Accept
Soil friction angle of layer 1 (ϕ_1)								
Training	-0.530	-1.961	1.961	Accept	1.006	-0.937	0.937	Accept
Testing	0.788	-1.961	1.961	Accept	0.978	-0.916	0.916	Accept
Validation	0.125	-1.961	1.961	Accept	1.025	-0.869	0.869	Accept
Soil friction angle of layer 2 (ϕ_2)								
Training	0.309	-1.961	1.961	Accept	0.997	-0.937	0.937	Accept
Testing	-0.970	-1.961	1.961	Accept	1.025	-0.916	0.916	Accept
Validation	0.475	-1.961	1.961	Accept	0.995	-0.869	0.869	Accept
Soil friction angle of layer 3 (ϕ_3)								
Training	1.227	-1.961	1.961	Accept	0.972	-0.937	0.937	Accept
Testing	-1.626	-1.961	1.961	Accept	1.059	-0.916	0.916	Accept
Validation	-0.596	-1.961	1.961	Accept	0.912	-0.869	0.869	Accept
Soil friction angle of layer 4 (ϕ_4)								
Training	0.174	-1.961	1.961	Accept	0.992	-0.937	0.937	Accept
Testing	-0.994	-1.961	1.961	Accept	1.021	-0.916	0.916	Accept
Validation	0.764	-1.961	1.961	Accept	1.050	-0.869	0.869	Accept
Soil friction angle of layer 5 (ϕ_5)								
Training	1.130	-1.961	1.961	Accept	0.992	-0.937	0.937	Accept
Testing	-1.373	-1.961	1.961	Accept	1.011	-0.916	0.916	Accept
Validation	-0.642	-1.961	1.961	Accept	0.946	-0.869	0.869	Accept
Soil friction angle of layer 6 (ϕ_6)								
Training	-0.473	-1.961	1.961	Accept	1.003	-0.937	0.937	Accept
Testing	0.935	-1.961	1.961	Accept	0.995	-0.916	0.916	Accept
Validation	-0.142	-1.961	1.961	Accept	0.996	-0.869	0.869	Accept
Soil friction angle of layer 7 (ϕ_7)								
Training	0.050	-1.961	1.961	Accept	1.003	-0.937	0.937	Accept
Testing	0.079	-1.961	1.961	Accept	0.985	-0.916	0.916	Accept
Validation	-0.184	-1.961	1.961	Accept	1.005	-0.869	0.869	Accept

Table C.2 The results of null hypothesis tests inputs and outputs. (*Continued*)

Model Variable and Data Sets	t-value	Lower Critical value	Upper Critical value	t-test	F-value	Lower Critical value	Upper Critical value	F-test
Soil friction angle of layer 8 (ϕ_8)								
Training	0.158	-1.961	1.961	Accept	0.985	-0.937	0.937	Accept
Testing	-1.015	-1.961	1.961	Accept	1.038	-0.916	0.916	Accept
Validation	0.809	-1.961	1.961	Accept	1.011	-0.869	0.869	Accept
Soil friction angle of layer 9 (ϕ_9)								
Training	-0.412	-1.961	1.961	Accept	1.016	-0.937	0.937	Accept
Testing	0.195	-1.961	1.961	Accept	0.974	-0.916	0.916	Accept
Validation	0.561	-1.961	1.961	Accept	0.997	-0.869	0.869	Accept
Soil friction angle of layer 10 (ϕ_{10})								
Training	0.861	-1.961	1.961	Accept	0.992	-0.937	0.937	Accept
Testing	-1.787	-1.961	1.961	Accept	1.024	-0.916	0.916	Accept
Validation	0.325	-1.961	1.961	Accept	0.969	-0.869	0.869	Accept
Soil thickness of layer 1 (h_1)								
Training	-0.027	-1.961	1.961	Accept	0.974	-0.937	0.937	Accept
Testing	-0.611	-1.961	1.961	Accept	1.070	-0.916	0.916	Accept
Validation	0.713	-1.961	1.961	Accept	0.986	-0.869	0.869	Accept
Soil thickness of layer 2 (h_2)								
Training	-0.152	-1.961	1.961	Accept	1.004	-0.937	0.937	Accept
Testing	-0.561	-1.961	1.961	Accept	0.989	-0.916	0.916	Accept
Validation	0.917	-1.961	1.961	Accept	1.054	-0.869	0.869	Accept
Soil thickness of layer 3 (h_3)								
Training	-0.755	-1.961	1.961	Accept	0.994	-0.937	0.937	Accept
Testing	1.393	-1.961	1.961	Accept	1.033	-0.916	0.916	Accept
Validation	-0.088	-1.961	1.961	Accept	0.941	-0.869	0.869	Accept
Soil thickness of layer 4 (h_4)								
Training	0.113	-1.961	1.961	Accept	1.008	-0.937	0.937	Accept
Testing	0.786	-1.961	1.961	Accept	0.983	-0.916	0.916	Accept
Validation	-1.097	-1.961	1.961	Accept	0.968	-0.869	0.869	Accept
Soil thickness of layer 5 (h_5)								
Training	0.346	-1.961	1.961	Accept	0.995	-0.937	0.937	Accept
Testing	-0.265	-1.961	1.961	Accept	1.042	-0.916	0.916	Accept
Validation	-0.365	-1.961	1.961	Accept	0.949	-0.869	0.869	Accept

Table C.2 The results of null hypothesis tests inputs and outputs. (*Continued*)

Model Variable and Data Sets	t-value	Lower Critical value	Upper Critical value	t-test	F-value	Lower Critical value	Upper Critical value	F-test
Soil thickness of layer 6 (h_6)								
Training	-0.151	-1.961	1.961	Accept	0.997	-0.937	0.937	Accept
Testing	-0.285	-1.961	1.961	Accept	1.021	-0.916	0.916	Accept
Validation	0.599	-1.961	1.961	Accept	0.921	-0.869	0.869	Accept
Soil thickness of layer 7 (h_7)								
Training	-0.445	-1.961	1.961	Accept	0.988	-0.937	0.937	Accept
Testing	1.558	-1.961	1.961	Accept	1.010	-0.916	0.916	Accept
Validation	-0.885	-1.961	1.961	Accept	0.978	-0.869	0.869	Accept
Soil thickness of layer 8 (h_8)								
Training	0.653	-1.961	1.961	Accept	1.000	-0.937	0.937	Accept
Testing	-0.795	-1.961	1.961	Accept	1.010	-0.916	0.916	Accept
Validation	-0.362	-1.961	1.961	Accept	0.972	-0.869	0.869	Accept
Soil thickness of layer 9 (h_9)								
Training	0.705	-1.961	1.961	Accept	0.990	-0.937	0.937	Accept
Testing	-0.890	-1.961	1.961	Accept	1.020	-0.916	0.916	Accept
Validation	-0.366	-1.961	1.961	Accept	0.980	-0.869	0.869	Accept
Footing width (B)								
Training	-0.760	-1.961	1.961	Accept	0.983	-0.937	0.937	Accept
Testing	0.552	-1.961	1.961	Accept	0.998	-0.916	0.916	Accept
Validation	0.862	-1.961	1.961	Accept	0.973	-0.869	0.869	Accept
Average bearing capacity of strip footing (q_{av})								
Training	1.280	-1.961	1.961	Accept	1.012	-0.937	0.937	Accept
Testing	-1.162	-1.961	1.961	Accept	0.984	-0.916	0.916	Accept
Validation	-1.124	-1.961	1.961	Accept	0.949	-0.869	0.869	Accept
\tilde{N}_c								
Training	-0.510	-1.961	1.961	Accept	0.972	-0.937	0.937	Accept
Testing	1.388	-1.961	1.961	Accept	1.037	-0.916	0.916	Accept
Validation	-0.550	-1.961	1.961	Accept	0.919	-0.869	0.869	Accept
\tilde{N}_ϕ								
Training	0.159	-1.961	1.961	Accept	0.964	-0.937	0.937	Accept
Testing	-0.019	-1.961	1.961	Accept	1.078	-0.916	0.916	Accept
Validation	-0.288	-1.961	1.961	Accept	1.121	-0.869	0.869	Accept

