DEBONDING MECHANISMS OF FIBRE REINFORCED POLYMER STRENGTHENED STEEL MEMBERS

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TABLE OF CONTENTS

| TABI | LE OF CONTENTS1 |
|------|--|
| LIST | OF TABLES |
| LIST | OF FIGURES9 |
| ABB | REVIATIONS AND NOMENCLATURES17 |
| ABS | TRACT19 |
| STAT | TEMENT OF ORIGINALITY20 |
| ACK | NOWLEDGEMENT21 |
| LIST | OF PUBLICATIONS 22 |
| СНА | PTER 1: INTRODUCTION23 |
| 1.1 | BACKGROUND |
| 1.2 | BOND-SLIP (τ - δ) RELATIONSHIP |
| 1.3 | INTERMEDIATE CRACK (IC) DEBONDING |
| 1.4 | FULL AND PARTIAL INTERACTION THEORY26 |
| 1.5 | SCOPE AND OBJECTIVES |
| 1.6 | THESIS OUTLINE27 |

| CHAP | TER 2 | : LITERATUR | E REVIEW | |
|------|---------------|-------------|--|----------|
| 2.1 | INTRO | DUCTION | | 28 |
| 2.2 | RESE | ARCH RELAT | TED TO DEBONDING OF FRP | 28 |
| 2.3 | <i>τ-δ</i> RE | ELATIONSHIP | | 30 |
| 2.4 | DEBC | NDING LOAD | D, P _{IC} | 31 |
| 2.5 | CRITI | CAL BOND LI | ENGTH | |
| 2.6 | BONE | TESTING MI | ETHODS IN FRP-STEEL | 37 |
| | 2.6.1 | LOAD INDIR | ECTLY APPLIED TO THE FRP AND THE STEE | EL |
| | | PLATE IN BE | EAM - NOZAKA, SHIELD AND HAJJAR (2005). | |
| | 2.6.2 | LOAD DIREC | CTLY APPLIED TO THE FRP | |
| | | 2.6.2.1 | DAMATTY AND ABUSHAGUR (2003) | |
| | | 2.6.2.2 | XIA AND TENG (2005) | 40 |
| | 2.6.3 | LOAD DIREC | CTLY APPLIED TO THE STEEL ELEMENT WIT | HOUT |
| | | A GAP | | |
| | | 2.6.3.1 | MILLER, CHAJES, MERTZ AND HASTINGS (2 | 2001) 42 |
| | | 2.6.3.2 | COLOMBI AND POGGI (2005) | |
| | | 2.6.3.3 | AL-EMRANI AND KLIGER (2006) | |
| | 2.6.4 | LOAD DIREC | CTLY APPLIED TO THE STEEL ELEMENT WIT | ΉA |
| | | GAP - COLO | MBI AND POGGI (2005) | |
| 2.7 | EXTR | ACTING THE | τ_{max} - δ_{max} RELATIONSHIP | 51 |
| 2.8 | PART | IAL-INTERAC | TION NUMERICAL METHOD OF BOND-SLIP | |
| | RELA | TIONSHIP | | 52 |
| 2.9 | CONC | LUSIONS | | 54 |
| | | | | |
| CHAP | TER 3 | : PUSH PULL | . TESTS | 57 |
| 3.1 | INTRO | DUCTION | | 57 |
| 3.2 | SPEC | IMENS | | 57 |
| 3.3 | TEST | SETUP | | 60 |
| 3.4 | INST | RUMENTATIO | N | 63 |
| 3.5 | MATE | RIAL PROPE | RTIES | 64 |

| 3.6 | TEST | RESULTS O | F SIKA SPECIMEN | 64 |
|-----|-------|------------|------------------------------------|------------|
| | 3.6.1 | SPECIMEN | SIKA 1 | 65 |
| | 3.6.2 | SPECIMEN | SIKA 2 | 67 |
| | 3.6.3 | SPECIMEN | SIKA 3 | 69 |
| | 3.6.4 | SPECIMEN | SIKA 4 | 71 |
| | 3.6.5 | SPECIMEN | SIKA 5 | 73 |
| | 3.6.6 | SUMMARY | ON SIKA SERIES TESTS | 75 |
| 3.7 | TEST | RESULTS O | F CIBA SPECIMEN | 77 |
| | 3.7.1 | SPECIMEN | CIBA 6 | 77 |
| | 3.7.2 | SPECIMEN | CIBA 7 | 79 |
| | 3.7.3 | SPECIMEN | CIBA 8 | 81 |
| | 3.7.4 | SPECIMEN | CIBA 9 | 83 |
| | 3.7.5 | SPECIMEN | CIBA 10 | 85 |
| | 3.7.6 | SUMMARY | ON CIBA SERIES TESTS | 87 |
| 3.8 | ADDI | TIONAL TES | TS WITH CIBA ADHESIVE | |
| | 3.8.1 | ADDITIONA | L NOT FULLY ANCHORED SPECIMENS | 90 |
| | | 3.8.1.1 | SPECIMEN CIBA 11 | 90 |
| | | 3.8.1.2 | SPECIMEN CIBA 13 | 91 |
| | | 3.8.1.3 | SPECIMEN CIBA 14 | 92 |
| | 3.8.2 | ADDITIONA | L FULLY ANCHORED SPECIMEN TESTS. | 92 |
| | | 3.8.2.1 | SPECIMEN CIBA 12 | 94 |
| | | 3.8.2.2 | SPECIMEN CIBA 15 | |
| | | 3.8.2.3 | SPECIMEN CIBA 16 | 101 |
| | | 3.8.2.4 | SPECIMEN CIBA 17 | 106 |
| | 3.8.3 | SUMMARY | ON ADDITIONAL TESTS WITH CIBA SPEC | CIMENS.109 |
| 3.9 | CON | CLUSIONS | | 112 |

CHAPTER 4: ANALYSIS ON THE DERIVATION OF τ - δ RELATIONSHIP....... 113

| 4.2 | PARTIAL-INTERACTION NUMERICAL MODELLING OF LOCAL AND | | | |
|-----|--|---------|--|--|
| | GLOBAL BOND CHARACTERISTICS OF FRP PLATED STEEL | JOINTS | | |
| | 113 | | | |
| | 4.2.1 DISCUSSION ON THE CRITICAL BOND LENGTH | 116 | | |
| 4.3 | τ_{max} FROM NOT FULLY ANCHORED CIBA SPECIMENS | 118 | | |
| 4.4 | P_{IC} FROM FULLY ANCHORED SPECIMENS AND δ_{max} FROM T | HE | | |
| | GENERIC EQUATIONS | 119 | | |
| 4.5 | δ_1 FROM PARTIAL-INTERACTION NUMERICAL MODELLING | 120 | | |
| 4.6 | COMPARISON BETWEEN EXPERIMENTAL RESULT WITH NU | MERICAL | | |
| | ANALYSIS | 120 | | |
| | 4.6.1 SPECIMEN SIKA 1 | 120 | | |
| | 4.6.2 SPECIMEN SIKA 2 | 121 | | |
| | 4.6.3 SPECIMEN SIKA 3 | 122 | | |
| | 4.6.4 SPECIMEN SIKA 4 | 123 | | |
| | 4.6.5 SPECIMEN SIKA 5 | 124 | | |
| | 4.6.6 SPECIMEN CIBA 6 | 125 | | |
| | 4.6.7 SPECIMEN CIBA 7 | 126 | | |
| | 4.6.8 SPECIMEN CIBA 7 | 127 | | |
| | 4.6.9 SPECIMEN CIBA 10 | 128 | | |
| | 4.6.10 SPECIMEN CIBA 11 | 129 | | |
| | 4.6.11 SPECIMEN CIBA 13 | 130 | | |
| | 4.6.12 SPECIMEN CIBA 14 | 131 | | |
| | 4.6.13 SPECIMEN CIBA 16 | 132 | | |
| | 4.6.14 SPECIMEN CIBA 17 | 133 | | |
| 4.7 | ANALYSIS OF PUBLISHED RESULTS (XIA AND TENG 2005) | 134 | | |
| | 4.7.1 τ_{max} - δ_{max} FROM EXPERIMENT RESULTS | 135 | | |
| 4.8 | COMPARISON OF XIA AND TENG (2005) EXPERIMENTAL RE | SULT | | |
| | WITH CURRENT RESEARCH | | | |
| | 4.8.1 SPECIMEN A-1 | 137 | | |
| | 4.8.2 SPECIMEN A-2A | | | |
| | 4.8.3 SPECIMEN B-1 | | | |

| | 4.8.4 | SPECIMEN I | 3-2A140 |
|------|-------|-------------|---|
| 4.9 | SUMN | ARY ON THE | E NUMERICAL ANALYSIS BEST FIT CURVE 142 |
| 4.10 | CONC | LUSIONS | |
| | | | |
| CHAP | TER 5 | : STEEL COU | JPON TESTS144 |
| 5.1 | INTRO | DUCTION | |
| 5.2 | SPEC | IMENS | |
| 5.3 | TEST | SETUP | |
| 5.4 | INSTE | RUMENTATIC | N |
| 5.5 | MATE | RIAL PROPE | RTIES149 |
| 5.6 | TEST | RESULTS OI | F CW SPECIMENS 150 |
| | 5.6.1 | SPECIMEN | CW1150 |
| | 5.6.2 | SPECIMEN | CW2152 |
| | 5.6.3 | COMPARISO | ONS BETWEEN SPECIMENS CW1 AND CW2 154 |
| 5.7 | TEST | RESULTS OI | F VW SPECIMENS155 |
| | 5.7.1 | SPECIMEN | /W1155 |
| | | 5.7.1.1 | MIDDLE SECTION (0 mm) 157 |
| | | 5.7.1.2 | 10 mm FROM THE MIDDLE159 |
| | | 5.7.1.3 | 80 mm AND 110 mm FROM THE MIDDLE 161 |
| | | 5.7.1.4 | STRAINS ACROSS THE BOND LENGTH 163 |
| | 5.7.2 | SPECIMEN | /W2165 |
| | | 5.7.2.1 | MIDDLE SECTION (0 mm) 167 |
| | | 5.7.2.2 | 10 mm FROM THE MIDDLE168 |
| | | 5.7.2.3 | 80 mm AND 110 mm FROM THE MIDDLE 171 |
| | | 5.7.2.4 | STRAINS ACROSS THE BOND LENGTH 173 |
| 5.8 | CONC | CLUSIONS | |

CHAPTER 6: ANALYSIS OF THE DEBONDING MECHANISM IN FRP PLATED

| STEEL MEMBERS | | |
|---------------|--|-----|
| 6.1 | | 176 |

| 6.2 | PART | IAL-INTERACTION NUMERICAL METHODS FOR STEEL DUE TO |
|-----|-------|---|
| | AXIAL | FORCE ONLY |
| | 6.2.1 | MATERIAL PROPERTIES |
| | 6.2.2 | THE FORCES IN THE STEEL PLATE AND FRP 177 |
| | 6.2.3 | BOUNDARY CONDITION $\sigma_p=0$ AT THE END OF THE FRP PLATE |
| | | 178 |
| | 6.2.4 | BOUNDARY CONDITION ds/dx=s=0 ALONG THE FRP PLATE . 179 |
| | 6.2.5 | PARTIAL-INTERACTION NUMERICAL METHOD FOR |
| | | DEBONDING MECHANISM |
| 6.3 | DEBC | NDING MECHANISM |
| | 6.3.1 | PLATE END DEBONDING |
| | 6.3.2 | DEBONDING BETWEEN PLATE ENDS DUE TO STEEL YIELDING |
| | | 188 |
| | 6.3.3 | COMBINATION OF PLATE END DEBONDING AND DEBONDING |
| | | DUE TO YIELDING OF STEEL 191 |
| 6.4 | ANAL | YSIS OT TEST RESULTS193 |
| | 6.4.1 | SPECIMENS CW1 AND CW2193 |
| | 6.4.2 | SPECIMEN VW1196 |
| | 6.4.3 | SPECIMEN VW2 |
| 6.5 | COM | PARISON WITH PUBLISHED RESULTS |
| 6.6 | CON | CLUSIONS |

CHAPTER 7: DEVELOPMENT OF MOMENT-ROTATION CAPACITY

| NUM | ERICAL METHOD FOR FRP PLATED STEEL BEAM | 209 |
|-----|--|--------|
| 7.1 | INTRODUCTION | 209 |
| 7.2 | DEFINITION OF ROTATION CAPACITY | |
| 7.3 | PLASTIC MOMENT OF CONTINUOUS BEAMS | 212 |
| 7.4 | ROTATION CAPACITY OF BEAMS | 217 |
| 7.5 | PARTIAL-INTERACTION NUMERICAL METHOD FOR FRP PLA | TED I- |
| | SECTION STEEL BEAM | |

| 7.6 | NUMERICAL METHOD PROCEDURE | 221 |
|------|--|-----|
| 7.7 | DEBONDING MECHANISM | 223 |
| | 7.7.1 PLATE END DEBONDING | 224 |
| | 7.7.2 DEBONDING AT CENTRE DUE TO STEEL YIELDING | 227 |
| 7.8 | CONCLUSIONS | 227 |
| CUAT | | 220 |
| СПА | TER 8: SUMMARY AND CONCLUSIONS | 228 |
| 8.1 | INTRODUCTION | 228 |
| 8.2 | CONCLUSIONS ON THE PUSH PULL TESTS AND NUMERICAL | |
| | METHOD | 229 |
| 8.3 | CONCLUSIONS ON THE STEEL COUPON TESTS AND NUMERI | CAL |
| | METHOD | 230 |
| 8.4 | SUGGESTION FOR FUTURE WORK | 231 |
| | | |
| REFE | RENCES | 232 |
| | | |
| APPE | ENDIX A | 237 |
| | | |
| APPE | ENDIX A | 237 |
| | | |
| | | 220 |
| AFFL | | 230 |
| | | |
| APPE | ENDIX C | 239 |
| | | |
| APPE | ENDIX D | 240 |
| | | |
| | | 241 |

LIST OF TABLES

| Table 3.1 | Material and geometric properties of the FRP plate | 59 |
|-----------|--|----|
| Table 3.2 | Material properties of adhesive | 64 |
| Table 3.3 | Material properties of FRP for the additional CIBA tests | 89 |
| Table 3.4 | Loading and unloading procedure for the additional CIBA tests | 94 |
| Table 3.5 | Failure load and shear stress for not fully bonded specimens 1 | 10 |
| Table 4.1 | Critical bond length comparison1 | 18 |
| Table 5.1 | Geometrical properties of test specimens1 | 46 |
| Table 5.2 | Material properties of the steel plate1 | 50 |
| | | |

LIST OF FIGURES

| Figure 1.1 Bilin | ear bond-slip relationship | 24 |
|------------------|--|----|
| Figure 1.2 Inter | rmediate crack debonding mechanism | 25 |
| Figure 1.3 Deg | ree of interaction | 26 |
| Figure 2.4 | Local bond-slip model | 30 |
| Figure 2.5 | Interfacial stress distribution and propagation of debonding for a | |
| | large bond length | 32 |
| Figure 2.6 | Typical theoretical load-displacement curve | 33 |
| Figure 2.7 | Definition of IC debonding failure plane (cross sectional view of | |
| | plate) | 36 |
| Figure 2.8 | Experimental test setup and dimensions (Nozaka et al. 2005) | 38 |
| Figure 2.9 | Comparison of analytical and experimental test results (Nozaka et | |
| | al. 2005) | 39 |
| Figure 2.10 | Schematic of the conducted shear lap tests (Damatty and | |
| | Abushagur 2003) | 40 |
| Figure 2.11 | Pull test specimen setup from Xia and Teng | 41 |
| Figure 2.12 | Shear stress distribution | 42 |
| Figure 2.13 | Schematic of bond test specimen (Miller et al. 2001) | 43 |
| Figure 2.14 | Comparison of measured and computed strain along FRP (Miller | |
| | et al. 2001) | 44 |
| Figure 2.15 | Schematic of bond test specimen (Colombi and Poggi 2006) | 45 |
| Figure 2.16 | Comparison of measured and computed strain along FRP | |
| | (specimen without a gap) (Colombi and Poggi 2006) | 46 |
| Figure 2.17 | Schematic illustration of the principal load effects in a steel beam | |
| | glued with FRP (AI-Emrani and Kliger 2006) | 47 |
| Figure 2.18 | Test specimen for pull test incorporating steel yielding (AI-Emrani | |
| | and Kliger 2006) | 48 |
| Figure 2.19 | Predicted stress variations along the bonded length of Specimen | |
| | A12 (AI-Emrani and Kliger 2006) | 49 |
| Figure 2.20 | Double lap joint specimen (Colombi and Poggi 2006) | 50 |

| Figure 2.21 | Comparison of measured and computes strain along FRP | | |
|-------------|---|----|--|
| | (specimen with a gap) (Colombi and Poggi 2006) | 50 | |
| Figure 2.22 | Graphical representation of numerical analysis (Haskett et al. | | |
| | 2007) | 53 | |
| Figure 2.23 | Typical pull test setup | 55 | |
| Figure 2.24 | Typical pull test with steel yielding setup | 56 | |
| Figure 3.1 | Typical pull test setup | 58 | |
| Figure 3.2 | Ball bearings set on the steel surface | 59 | |
| Figure 3.3 | Force applied on FRP-to-steel | 60 | |
| Figure 3.4 | Test rig with the specimen | 61 | |
| Figure 3.5 | Location of steel plate for restraining and aluminium plate for | | |
| | LVDT's restraint | 62 | |
| Figure 3.6 | Aluminium plate as a grip | 62 | |
| Figure 3.7 | Detail instrumentations of the specimen | 63 | |
| Figure 3.8 | Failure mode of specimen SIKA 1 | 66 | |
| Figure 3.9 | Global P- Δ for specimen SIKA 1 | 67 | |
| Figure 3.10 | Failure mode of specimen SIKA 2 | 68 | |
| Figure 3.11 | Global P- Δ for specimen SIKA 2 | 69 | |
| Figure 3.12 | Failure mode of specimen SIKA 3 | 70 | |
| Figure 3.13 | Global P- Δ for specimen SIKA 3 | 71 | |
| Figure 3.14 | Failure mode of specimen SIKA 4 | 72 | |
| Figure 3.15 | Global P- Δ for specimen SIKA 4 | 73 | |
| Figure 3.16 | Failure mode of specimen SIKA 5 | 74 | |
| Figure 3.17 | Global P- Δ for specimen SIKA 5 | 75 | |
| Figure 3.18 | Global P- Δ for specimen SIKA series | 76 | |
| Figure 3.19 | Failure mode of specimen CIBA 6 | 78 | |
| Figure 3.20 | Global P- Δ for specimen CIBA 6 | 79 | |
| Figure 3.21 | Failure mode of CIBA 7 specimen | 80 | |
| Figure 3.22 | Global P- Δ for specimen CIBA 7 | 81 | |
| Figure 3.23 | Failure mode of specimen CIBA 8 | 82 | |

| Figure 3.24 | Global P- Δ for specimen CIBA 8 | 83 |
|-------------|--|----------|
| Figure 3.25 | Failure mode of specimen CIBA 9 | 84 |
| Figure 3.26 | Global P- Δ for specimen CIBA 9 | 85 |
| Figure 3.27 | Failure mode of specimen CIBA 10 | 86 |
| Figure 3.28 | Global P- Δ for specimen CIBA 10 | 87 |
| Figure 3.29 | Global P- Δ for specimen CIBA series | 88 |
| Figure 3.30 | Global P- Δ for specimen CIBA 11 | 90 |
| Figure 3.31 | Global P- Δ for specimen CIBA 13 | 91 |
| Figure 3.32 | Global P- Δ for specimen CIBA 14 | 92 |
| Figure 3.33 | Detail instrumentations of the additional specimen | 93 |
| Figure 3.34 | Failure mode of specimen CIBA 12 | 96 |
| Figure 3.35 | Global P- Δ for specimen CIBA 12 | 97 |
| Figure 3.36 | Local τ - δ for specimen CIBA 12 | 97 |
| Figure 3.37 | Failure mode of specimen CIBA 15 | 99 |
| Figure 3.38 | Global P- Δ for specimen CIBA 15 | 100 |
| Figure 3.39 | Local τ - δ for specimen CIBA 15 | 101 |
| Figure 3.40 | Failure mode of specimen CIBA 16 | 103 |
| Figure 3.41 | Global P- Δ for specimen CIBA 16 | 104 |
| Figure 3.42 | Local τ - δ for specimen CIBA 16 | 104 |
| Figure 3.43 | Local τ - δ for specimen CIBA 16 (strain gauge 25 mm) | 105 |
| Figure 3.44 | Failure mode of specimen CIBA 17 | 107 |
| Figure 3.45 | Global <i>P-</i> ⊿ for specimen CIBA 17 | 108 |
| Figure 3.46 | Local τ - δ for specimen CIBA 17 | 109 |
| Figure 3.47 | Global <i>P-</i> ^{<i>A</i>} curve for the not fully bonded specimens | 110 |
| Figure 3.48 | Local τ - δ for specimen CIBA 16 and CIBA 17 calculated f | rom |
| | strain gauge 25 mm | 111 |
| Figure 4.1 | Graphical representation of the numerical analysis for FRI | P plated |
| | steel joints | 115 |
| Figure 4.2 | Influence of δ_1 to the global load–slip (τ - Δ) response | 116 |

| Figure 4.3 | Critical bond length analysis of specimen CIBA117 |
|-------------|---|
| Figure 4.4 | Bond stress distribution for a not fully anchored embedment119 |
| Figure 4.5 | Comparison between experimental and numerical P- Δ curves of |
| | specimen SIKA 1121 |
| Figure 4.6 | Comparison between experimental and numerical P- Δ curves of |
| | specimen SIKA 2122 |
| Figure 4.7 | Comparison between experimental and numerical P- Δ curves of |
| | specimen SIKA 3123 |
| Figure 4.8 | Comparison between experimental and numerical P- Δ curves of |
| | specimen SIKA 4124 |
| Figure 4.9 | Comparison between experimental and numerical P- Δ curves of |
| | specimen SIKA 5125 |
| Figure 4.10 | Comparison between experimental and numerical P- Δ curves of |
| | specimen CIBA 6126 |
| Figure 4.11 | Comparison between experimental and numerical P- Δ curves of |
| | specimen CIBA 7127 |
| Figure 4.12 | Comparison between experimental and numerical P- Δ curves of |
| | specimen CIBA 9128 |
| Figure 4.13 | Comparison between experimental and numerical P- Δ curves of |
| | specimen CIBA 10129 |
| Figure 4.14 | Comparison between experimental and numerical P- Δ curves of |
| | specimen CIBA 11130 |
| Figure 4.15 | Comparison between experimental and numerical P- Δ curves of |
| | specimen CIBA 13131 |
| Figure 4.16 | Comparison between experimental and numerical P- Δ curves of |
| | specimen CIBA 14132 |
| Figure 4.17 | Comparison between experimental and numerical P- Δ curves of |
| | specimen CIBA 16133 |
| Figure 4.18 | Comparison between experimental and numerical P- Δ curves of |
| | specimen CIBA 17134 |

| Figure 4.19 | Shear stress distributions from Xia and Teng (2005) experiments 136 |
|-------------|---|
| Figure 4.20 | Comparison between experimental and numerical P- Δ curves of |
| | specimen A-1138 |
| Figure 4.21 | Comparison between experimental and numerical P- Δ curves of |
| | specimen A-2A139 |
| Figure 4.22 | Comparison between experimental and numerical P- Δ curves of |
| | specimen B-1140 |
| Figure 4.23 | Comparison between experimental and numerical P- Δ curves of |
| | specimen B-2A141 |
| Figure 5.1 | Shape and dimension of test specimen with a constant width (CW) |
| | steel plate144 |
| Figure 5.2 | Shape and dimension of test specimen with a varying width (VW) |
| | steel plate145 |
| Figure 5.3 | Test setup147 |
| Figure 5.4 | Strain gauges location of test specimen with a constant width |
| | (CW) steel plate |
| Figure 5.5 | Strain gauges location of test specimen with a varying width (VW) |
| | steel plate148 |
| Figure 5.6 | Stress-strain relationship of the steel149 |
| Figure 5.7 | Failure mode of CW1151 |
| Figure 5.8 | Experimental result for CW1152 |
| Figure 5.9 | Failure mode of CW2153 |
| Figure 5.10 | Experimental result for CW2154 |
| Figure 5.11 | Comparison of strains between CW1 and CW2155 |
| Figure 5.12 | Failure mode of VW1156 |
| Figure 5.13 | Experimental result for VW1 at 0mm158 |
| Figure 5.14 | Experimental result for VW1 at 0mm at debonding158 |
| Figure 5.15 | Experimental result for VW1 at 10mm from the middle160 |
| Figure 5.16 | Experimental result for VW1 at 10mm from the middle at |
| | debonding (right)160 |

| Figure 5.17 | Experimental result for VW1 at 10mm from the middle at | |
|---|--|--|
| | debonding (left)161 | |
| Figure 5.18 | Experimental result for VW1 at 80mm from the middle at | |
| | debonding162 | |
| Figure 5.19 | Experimental result for VW1 at 110mm from the middle at | |
| | debonding162 | |
| Figure 5.20 | Strains across the bonded length at different stages (VW1)164 | |
| Figure 5.21 | Failure mode of VW2166 | |
| Figure 5.22 | Experimental result for VW2 at 0mm. | |
| Figure 5.23 | Experimental result for VW2 at 0mm at debonding168 | |
| Figure 5.24 | Experimental result for VW2 at 10mm | |
| Figure 5.25 | Experimental result for VW2 at 10mm at debonding (right) | |
| Figure 5.26 | Experimental result for VW2 at 10mm at debonding (left) | |
| Figure 5.27 | Experimental result for VW2 at 80mm at debonding172 | |
| Figure 5.28 | Experimental result for VW2 at 110mm at debonding172 | |
| Figure 5.29 | Strains across the bonded length at different stages (VW2) 174 | |
| Figure 6.1 | Stress-strain relationship of steel and FRP177 | |
| Figure 6.2 | Forces in steel and FRP178 | |
| Figure 6.3 | Strain distribution of steel and FRP178 | |
| Figure 6.4 | Strain, slipstrain and slip distributions of FRP plated steel member180 | |
| Figure 6.5 Graphical representation of the numerical analysis for FRP | | |
| | steel members | |
| Figure 6.6 | Strain distribution of steel and FRP for plate end debonding185 | |
| Figure 6.7 | Slipstrain distribution of steel and FRP for plate end debonding186 | |
| Figure 6.8 | Slip distribution of steel and FRP for plate end debonding | |
| Figure 6.9 | Strain distribution of steel and FRP when $\varepsilon_x < \varepsilon_p$ at the middle187 | |
| Figure 6.10 | Steel strain distribution after steel yielding189 | |
| Figure 6.11 | FRP strain distribution after steel yielding189 | |
| Figure 6.12 | Slipstrain distribution after steel yielding190 | |
| Figure 6.13 | Slip distribution after steel yielding190 | |

| Figure 6.14 | Bond stress distribution after steel yielding197 | 1 |
|-------------|---|----|
| Figure 6.15 | Full and partial interaction regions of FRP plated steel member 192 | 2 |
| Figure 6.16 | Numerical load-strain for CW1 and CW2193 | 3 |
| Figure 6.17 | Numerical load-strain for CW1195 | 5 |
| Figure 6.18 | Numerical bond stress for CW1 at steel yielding | 5 |
| Figure 6.19 | Steel load-strain comparison for VW1196 | 3 |
| Figure 6.20 | FRP load-strain comparison for VW1197 | 7 |
| Figure 6.21 | Numerical slip at different stages of loading (VW1) | 3 |
| Figure 6.22 | Numerical bond stress at different stages of loading (VW1) | 3 |
| Figure 6.23 | Peak numerical bond stress at debonding (VW1) | 9 |
| Figure 6.24 | Steel load-strain comparison for VW2200 |) |
| Figure 6.25 | FRP load-strain comparison for VW2207 | 1 |
| Figure 6.26 | Numerical and experimental load-axial stress comparison for | |
| | specimen A12204 | 1 |
| Figure 6.27 | Numerical and experimental load-axial stress comparison for | |
| | specimen B12204 | 4 |
| Figure 6.28 | Numerical and experimental load-axial stress comparison for | |
| | specimen B17205 | 5 |
| Figure 6.29 | Numerical and FEM shear stress comparison across the bonded | |
| | length206 | 3 |
| Figure 6.30 | Numerical and experimental shear stress comparison at plate end.2 | 07 |
| Figure 7.1 | Rotation capacity definition by ASCE212 | 1 |
| Figure 7.2 | Definition of rotation capacity based on normalized moment- | |
| | rotation relationship212 | 1 |
| Figure 7.3 | Standard beams for a continuous beam213 | 3 |
| Figure 7.4 | Characteristic points in a beam214 | 4 |
| Figure 7.5 | Locations of inflection and maximum moment points in standard | |
| | beams216 | 3 |
| Figure 7.6 | Moment rotation curve for SB1219 | 9 |
| Figure 7.7 | Moment rotation curve for SB2 |) |

| Figure 7.8 | Graphical representation of the numerical analysis for FRP plated | |
|-------------|---|-----|
| | steel section | 223 |
| Figure 7.9 | Specimen for the numerical procedure | 224 |
| Figure 7.10 | Strain distribution of steel and FRP at bottom flange | 225 |
| Figure 7.11 | Slipstrain distribution of steel and FRP at bottom flange | 225 |
| Figure 7.12 | Slip distribution of steel and FRP at bottom flange | 226 |
| Figure 7.13 | Plate end debonding at 150 mm bond length | 226 |

ABBREVIATIONS AND NOMENCLATURES

- τ bond stress
- χ curvature
- \varDelta global slip
- Δ_s change of slip
- δ slip
- ε strain
- δ_1 slip at maximum bond stress
- θ_1 theoretical rotation when full plastic capacity is achieved
- θ_2 rotation when moment capacity drops below M_p
- (AE)_p axial rigidity of FRP
- $(AE)_s$ axial rigidity of steel
- (EA)_c axial rigidity of the concrete
- (EA)_p axial rigidity of the FRP plate
- θ_h plastic rotation
- θ_{hm} plastic rotation at maximum moment
- τ_{max} maximum bond stress
- δ_{max} maximum slip
- $\delta_{max,cal}$ calculated maximum slip
- $\tau_{max,exp}$ experimental bond stress
- $\delta_{max,exp}$ experimental maximum slip
- θ_p elastic rotation
- ε_p FRP strain
- $\hat{\varepsilon}_s$ steel strain
- ε_u steel strain at ultimate rotation
- ε_{sh} strain hardening
- θ_u ultimate rotation
- $\varepsilon_{\rm Y}$ yield strain,
- A_p area of FRP
- \vec{B} bond force
- b width
- b_{ρ} FRP width
- CDC critical diagonal crack
- d depth
- d_x segment length
- *E_p* Young's Modulus of FRP
- *E*_s Young's Modulus of steel
- F force
- FRP Fibre Reinforced Polymer
- *f*_{sh} Strain hardening stress
- *f_y* Yield stress
- *G*_f fracture energy

- h height
- IC interfacial crack
- J geometry of the interface debonding failure plane
- *L_{crit}* critical bond length
- *L_p* length of embedment
- I_p length of full plastic zone
- *L_{per}* perimeter length
- *I*_{pf} length of flange plastic zone
- *M* moment
- M_{p} plastic moment
- *M_{pf}* plastic moment at flange
- *M_u* ultimate moment
- P load
- PE plate end
- P_{IC} debonding load
- P_{IC,exp} experimental interfacial crack load
- P_L load increment
- *P*_{UL} load decrement
- P_{y.} yield load
- *q* uniform distributed load
- *R* rotation capacity
- t_p thickness of FRP
- $\dot{t}_{\rm s}$ Steel thickness
- *x_i* distance to inflection point
- *x_m* distance to plastic moment

ABSTRACT

Applying Fibre Reinforce Polymer (FRP) to steel structures has been proved to be an effective method of strengthening. Experimentally, ageing steel structures such as bridge decks and composite beams which have been strengthened with FRP have shown lifetime extension and enhanced strength. Numerically, different approaches have been carried out to quantify the relationship between FRP and steel members in regard to the observance of the experimental works.

This thesis contributes in term of quantifying the debonding mechanism of FRP strengthened steel members. First, a procedure in the derivation of the bond-slip $(\tau - \sigma)$ relationship is presented by combining the results of the experimental work with a numerical method developed specifically for this purpose. Secondly, the debonding mechanisms of FRP strengthened steel plates due to the yielding of steel is established by experimental and numerical works. Finally, a numerical method was developed to quantify the plate end debonding of a simply supported steel beam.

A total of seventeen pull tests with different types of FRP lengths and adhesives were tested to quantify the (τ - σ) relationship. Another four steel plate tests were carried out to study the debonding mechanism of FRP allowing for the steel to yield. Three different numerical methods were developed to analyse the results obtained experimentally.

STATEMENT OF ORIGINALITY

This work contains no material which has been accepted for the award of any other degree or diploma in any university or other tertiary institution to Ibrisam Akbar, and, to the best of my knowledge and belief, contains no material previously published or written by another person except where due reference has been made in the text.

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LIST OF PUBLICATIONS

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